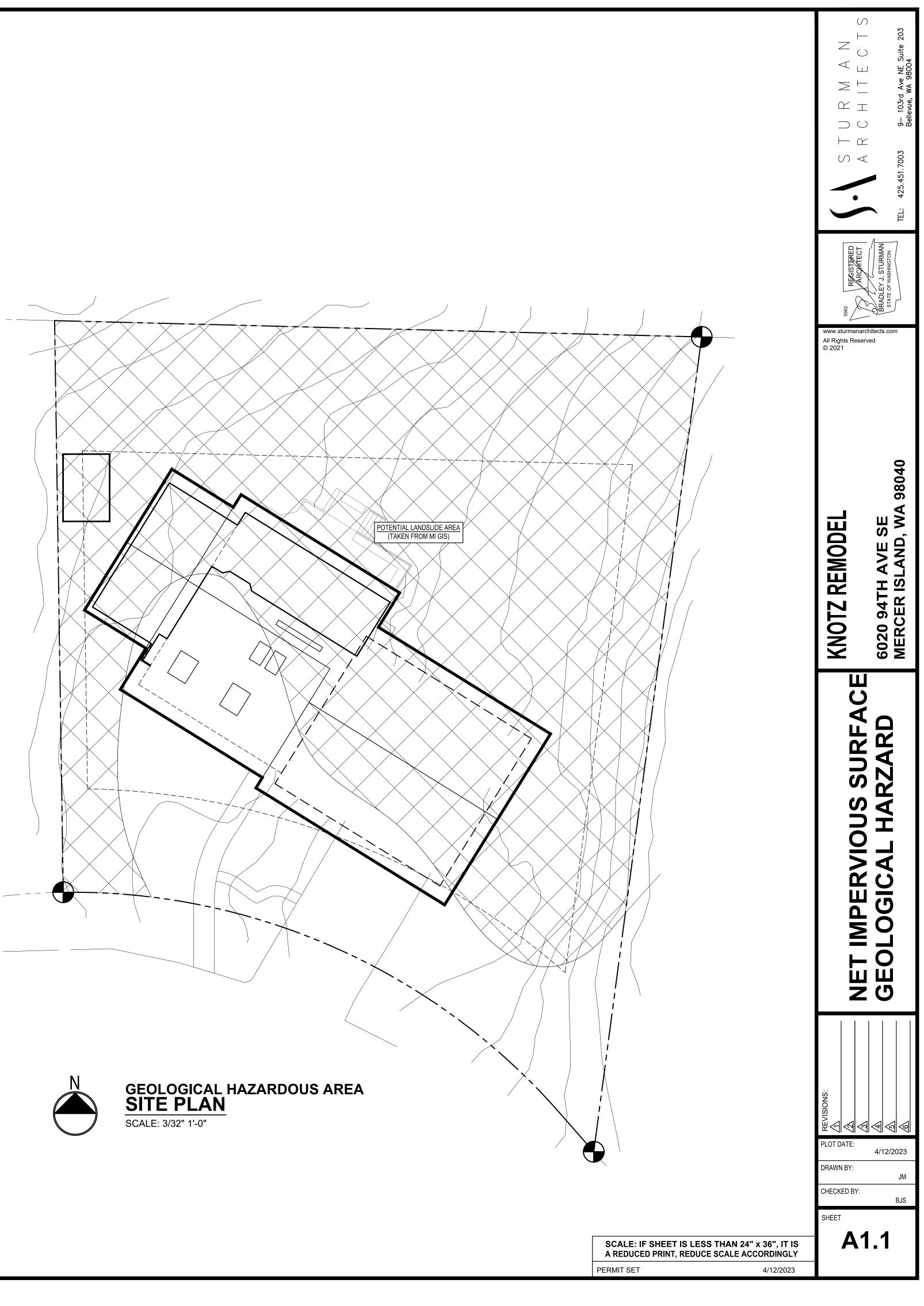
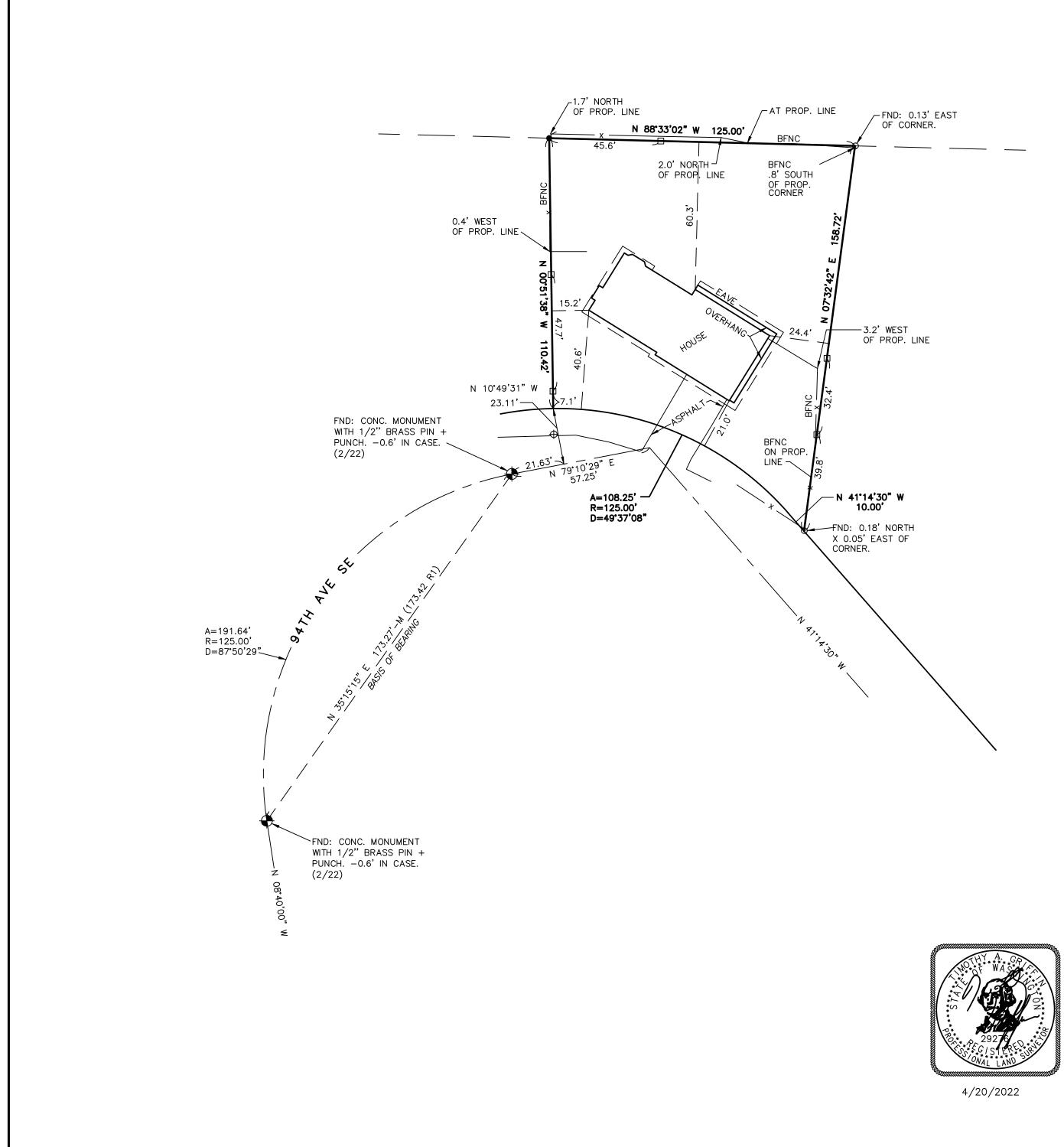


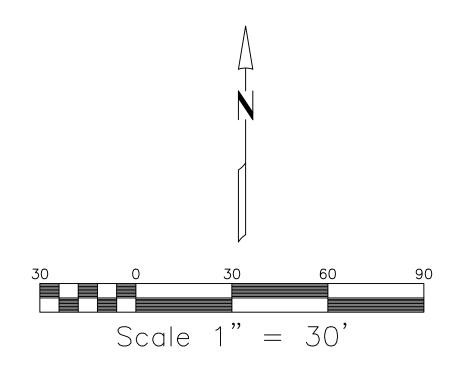
IMPERVIOUS SURFACE PLAN

SCALE: 1/8" 1'-0"









<u>MERIDIAN</u>

PLAT OF TIMBERLAND NO. 7

BASIS OF BEARING

AS SHOWN

<u>LEGEND</u>

- SET 1/2" X 24" REBAR WITH 1 3/4" PLASTIC CAP STAMPED "TYEE LS 29276"
- SET HUB ON LINE
- FOUND MAGNETIC NAIL WITH WASHER "PACE ENG." 10.55'
- SOUTH X 0.02' OF COMPUTED CORNER.
- FOUND 1/2" REBAR + CAP "GEO.-D LS 15025
- (R) REFERENCE DISTANCE
- (M) MEASURED DISTANCE

BFNC BOARD FENCE

EQUIPMENT & PROCEDURES

A 5" ELECTRONIC TOTAL STATION WAS USED FOR THIS FIELD TRAVERSE SURVEY. ACCURACY MEETS OR EXCEEDS W.A.C. 332–130–090.

REFERENCES

1. THE PLAT OF TIMBERLAND NO. 7, AS RECORDED IN VOLUME 73 OF PLATS, PAGES 90–91, RECORDS OF KING COUNTY, WASHINGTON.

LEGAL DESCRIPTION

PER STATUTORY WARRANTY DEED, RECORDING NO. 20170531000280, RECORDS OF KING COUNTY, WASHINGTON.

LOT 3, BLOCK 3, TIMBERLAND NO. 7, ACCORDING TO THE PLAT THEREOF RECORDED IN VOLUME 73 OF PLATS, PAGES 90 AND 91, IN KING COUNTY, WASHINGTON. SITUATE IN THE COUNTY OF KING, STATE OF WASHINGTON.

PARCEL NUMBER 8651200190

SW1/4, SE1/4, SEC. 19, T. 24 N., R. 5 E., W.M. MERCER ISLAND, WASHINGTON

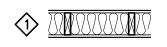
	PROFESSIONAL LAND SUF MIDVALE AVE N, STE 107. SHORELINE WA. 98133	VEYORS
SCALE: 1"=30'		drawn by: RG
DATE: 4/15/22		CHECK BY: TG
	HADRIAN KNOTZ	
6020 94TH AVE SE	MERCER ISLAN	D, WASHINGTON 98040
Ś	SITE PLAN	drawing number 22021
SW1/4, SE1/	/4, SEC. 19, T. 24 N., F	R. 5 E., W.M.

WALL PARTITION TYPES: N.T.S. (SEE STRUCTURAL SHEETS FOR SHEARWALLS.

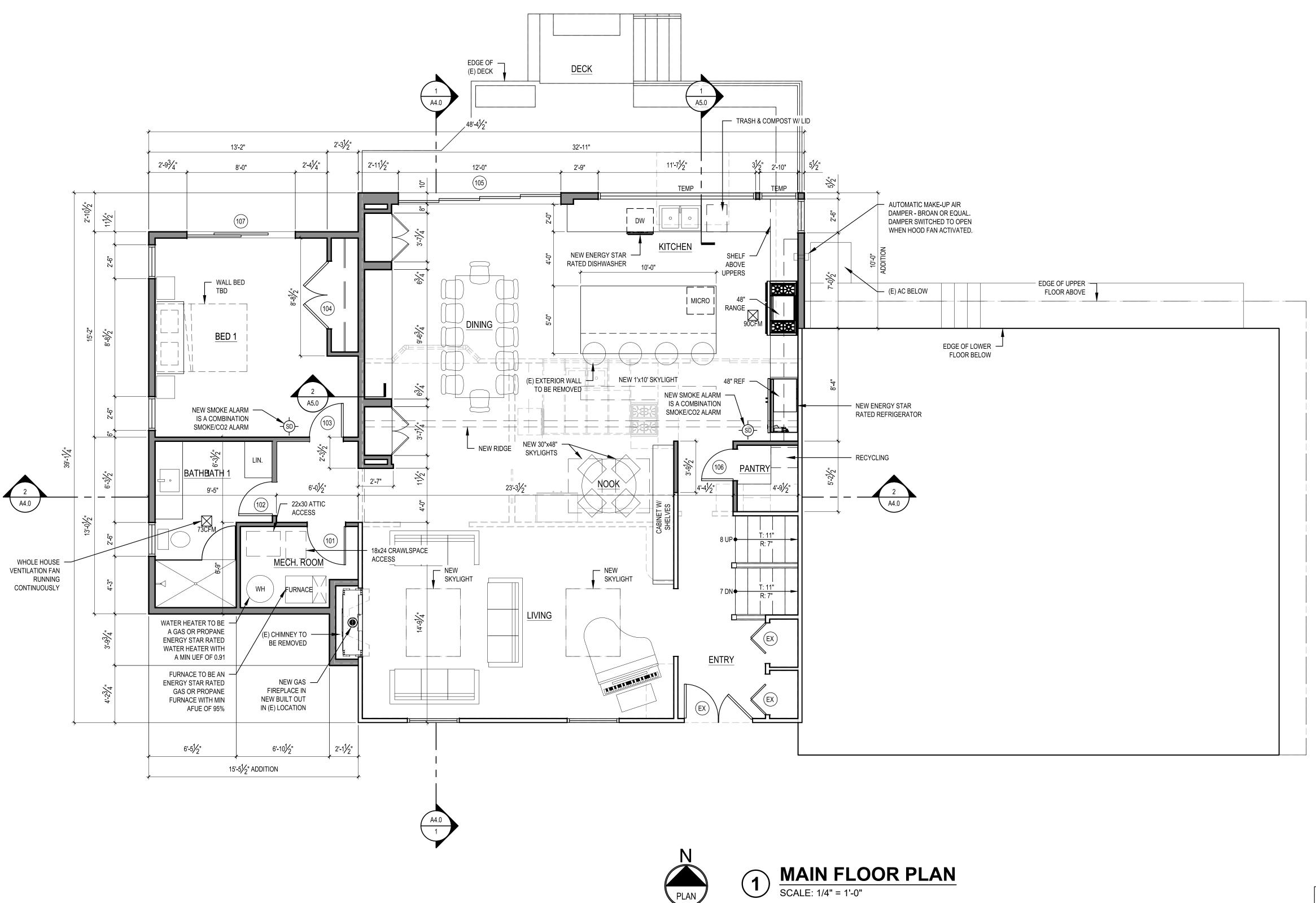
<u> []]]]</u>]]]]]]]]]]]]]]]]]]]]]]]]]]]]]]]]	TYPICAL EXTERIOR WALL EXTERIOR WALL FINISH o/ (2) LAYERS 60# BLDG. PAPER o/ 1/2" CDX PLYWOOD o/ 2x6 WOOD STUDS AT 16" O.C. w/ 1/2" GYPSUM WALLBOARD AT INTERIOR. PROVIDE R-21 BATT
	INSULATION EXCEPT AROUND GARAGE. <u>TYPICAL INTERIOR PARTITION</u> U.N.O. ALL INTERIOR WALL SHALL BE 2x4 WOOD STUDS @ 16" O.C. w/ 1/2" GYPSUM WALLBOARD EACH SIDE.

TYPICAL FURRED WALL

2" AIRSPACE, 2x4 P.T. WOOD STUDS @ 16" O.C. w/ 1/2" GYPSUM WALLBOARD AT INTERIOR. PROVIDE R-21 BATT INSULATION.



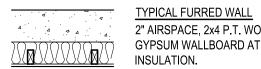
<u>1HR. FIRE RATED WALL</u> 5/8" THK GWB, TYPE 'X' O/ 2X6 WD STUDS @ 16" O.C. PANELS NAILED 7" O.C.-1 7/8" CEM CTD NAILS- JOINTS EXP OR FIN - PERIM CAULKED- UL DES U305 & U314- JOINTS FIN



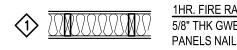
SCALE: IF SHEET IS LESS THAN 24" x 36", IT IS
A REDUCED PRINT, REDUCE SCALE ACCORDINGLYPERMIT SET4/12/2023

WALL PARTITION TYPES: N.T.S. (SEE STRUCTURAL SHEETS FOR SHEARWALLS.

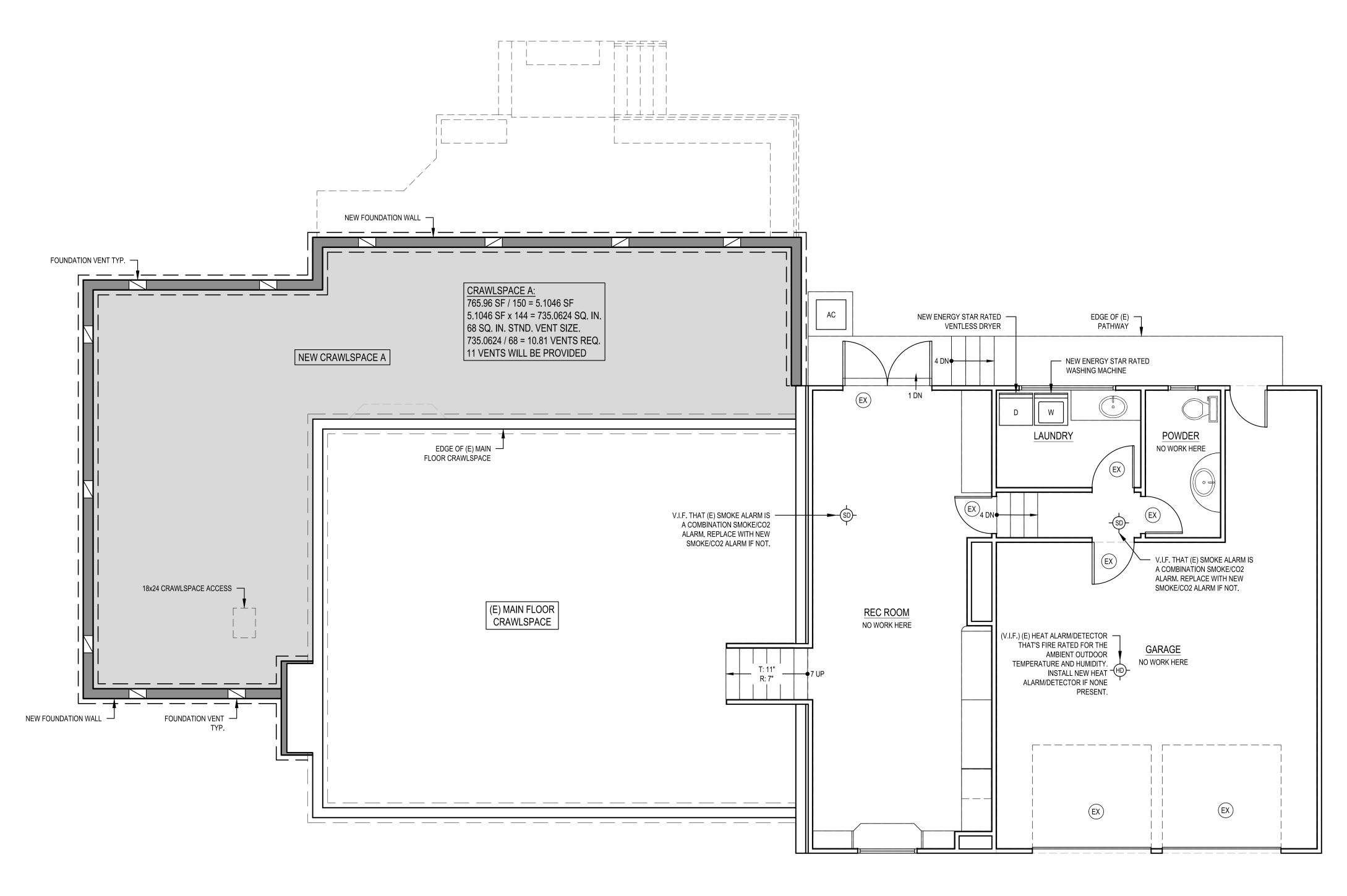
<u> </u>	TYPICAL EXTERIOR WALL EXTERIOR WALL FINISH o/ (2) LAYERS 60# BLDG. PAPER o/ 1/2" CDX PLYWOOD o/ 2x6 WOOD STUDS AT 16" O.C. w/ 1/2" GYPSUM WALLBOARD AT INTERIOR. PROVIDE R-21 BATT INSULATION EXCEPT AROUND GARAGE.
	TYPICAL INTERIOR PARTITION U.N.O. ALL INTERIOR WALL SHALL BE 2x4 WOOD STUDS @ 16" O.C. w/ 1/2" GYPSUM WALLBOARD EACH SIDE.



2" AIRSPACE, 2x4 P.T. WOOD STUDS @ 16" O.C. w/ 1/2" GYPSUM WALLBOARD AT INTERIOR. PROVIDE R-21 BATT INSULATION.



<u>1HR. FIRE RATED WALL</u> 5/8" THK GWB, TYPE 'X' O/ 2X6 WD STUDS @ 16" O.C. PANELS NAILED 7" O.C.-1 7/8" CEM CTD NAILS- JOINTS EXP OR FIN - PERIM CAULKED- UL DES U305 & U314- JOINTS FIN





All Rig	A R C H I T E C T S A R C H I T E C T S A R C H I T E C T S A R C H I T E C T S		TEL: 425.451.7003 9– 103rd Ave NE Suite 203 Bellevue, WA 98004
		6020 94TH AVE SE	MERCER ISLAND, WA 98040
MAIN FLOOR CRAWLSPACE	LOWER FLOOR PLAN		
SHEE	N BY: KED BY:	4/12/2	023 JM BJS

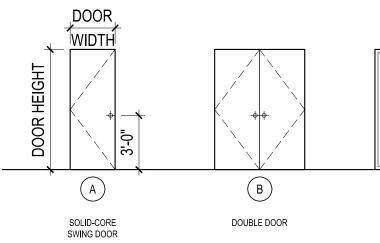
CODE REQUIREM	ENT			CALCULATIONS	5									ACTUAL	
DESCRIPTION	SF AREA	REQ. VE	NTING		VENT TYPE			VENT L.F.		TOTAL		SF CONVERT.		80% EFF	
		PER SF	AREA				x] =	VENT AREA	Х	1/144	x	FACTOR	ΤΟΤΑΙ
		150	300	RIDGE	GABLE	EAVE			1	SQ. IN.					
						18 SQ.IN./FT.		133.9		2410.2		16.74		13.39	21.25
						1.5x1.0" VENT			1						
	3,123	20.92		12 SQ.IN/FT.				75.2	1	902.4		6.27		5.01	
ROOF A	5,125	20.82		CONTINUOUS					1						
					256 SQ. IN			2	1	512		3.56		2.84	
					24x24" VENT		1		1						
						10 SQ.IN./FT.		28.9		520.2		3.61		2.89	3.99
						1.5x1.0" VENT	1		1						
	F40	2.60		12 SQ.IN/FT.			1	16.5	1	198		1.38		1.10	
ROOF B	540	3.60		CONTINUOUS					1						
							1		1	0		0.00		0.00	

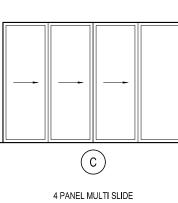
-

D

SLIDING DOOR

DOOR TYPES:



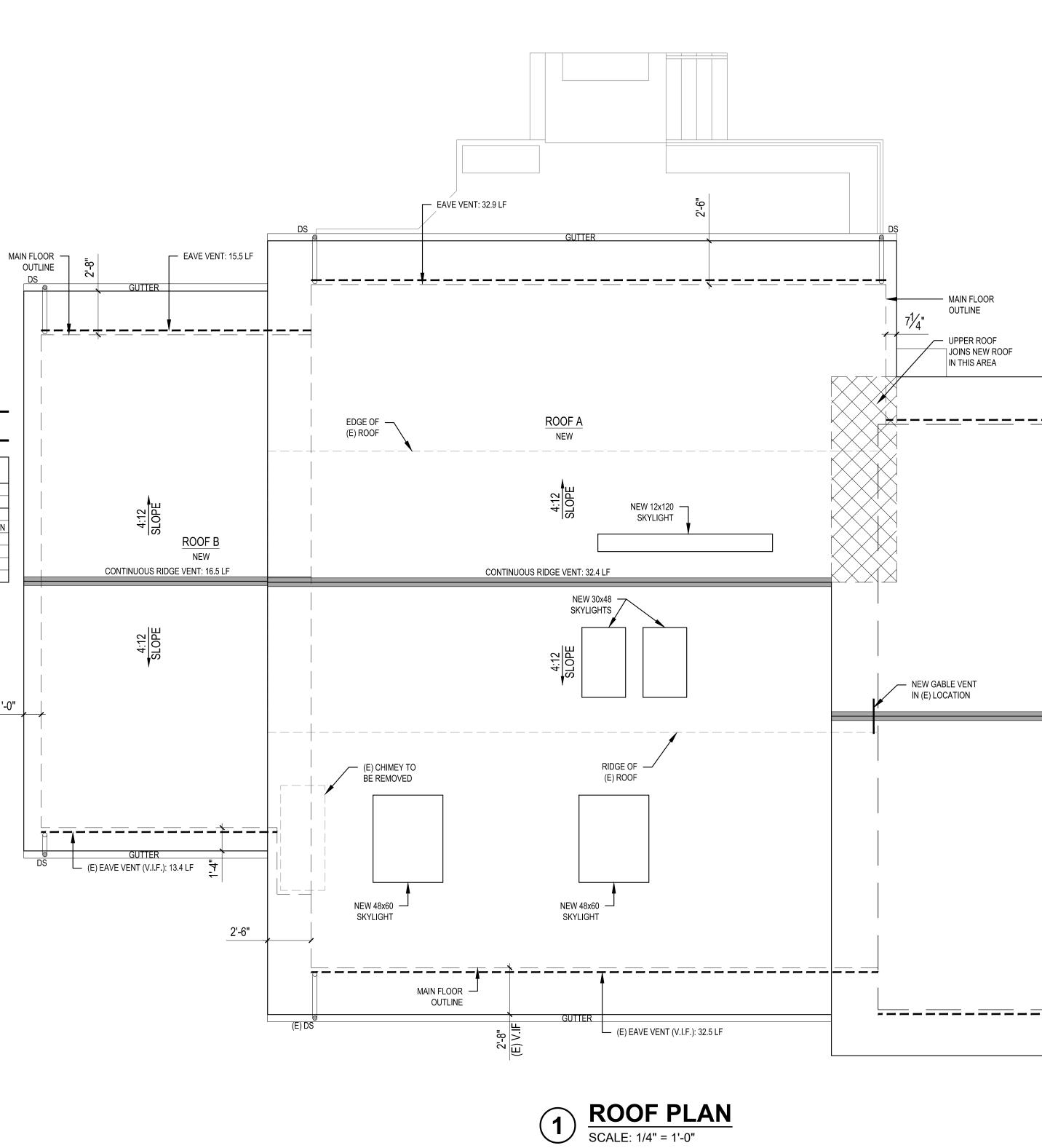


DOOR SCHEDULE

DOOR	LOCATION	SIZE	SIZE	DOOR	TEMP.	DOOR	DOOR	U-VAL.	NFRC	REMARKS
NO.		WIDTH	HEIGHT	TYPE	GLASS	FIN.	THK.	(MIN.)	CERT.	
MA	IN FLOOR									
101	MECH ROOM	2'-10"	8'-0"	Α	-	-	1-1/4"	-	Y	
102	BATH 1	2'-6"	8'-0"	Α	-	-	1-1/4"	-	Y	
103	BEDROOM 1	2'-6"	8'-0"	А	-	-	1-1/4"	-	Y	
104	BEDROOM 1	5'-0"	8'-0"	В	-	-	1-1/4"	-	Y	
105	DINING ROOM	12'-0"	8'-0"	С	Y	-	1-3/4"	.28	Y	
106	PANTRY	2'-6"	8'-0"	А	-	-	1-1/4"	-	Y	
107	BEDROOM 1	8'-0"	8'-0"	D	Y	-	1-1/4"	.28	Y	

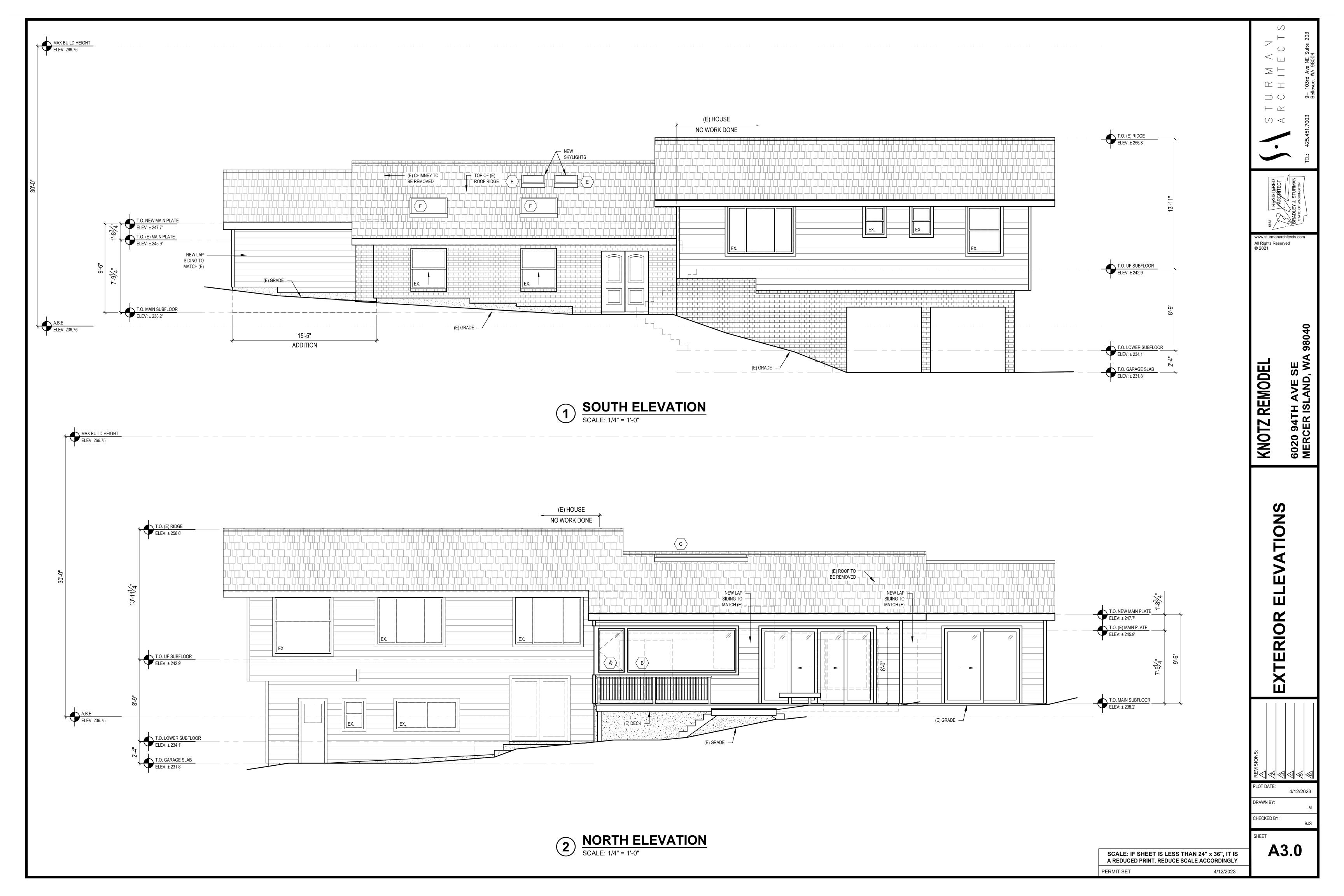
WINDOW SCHEDULE

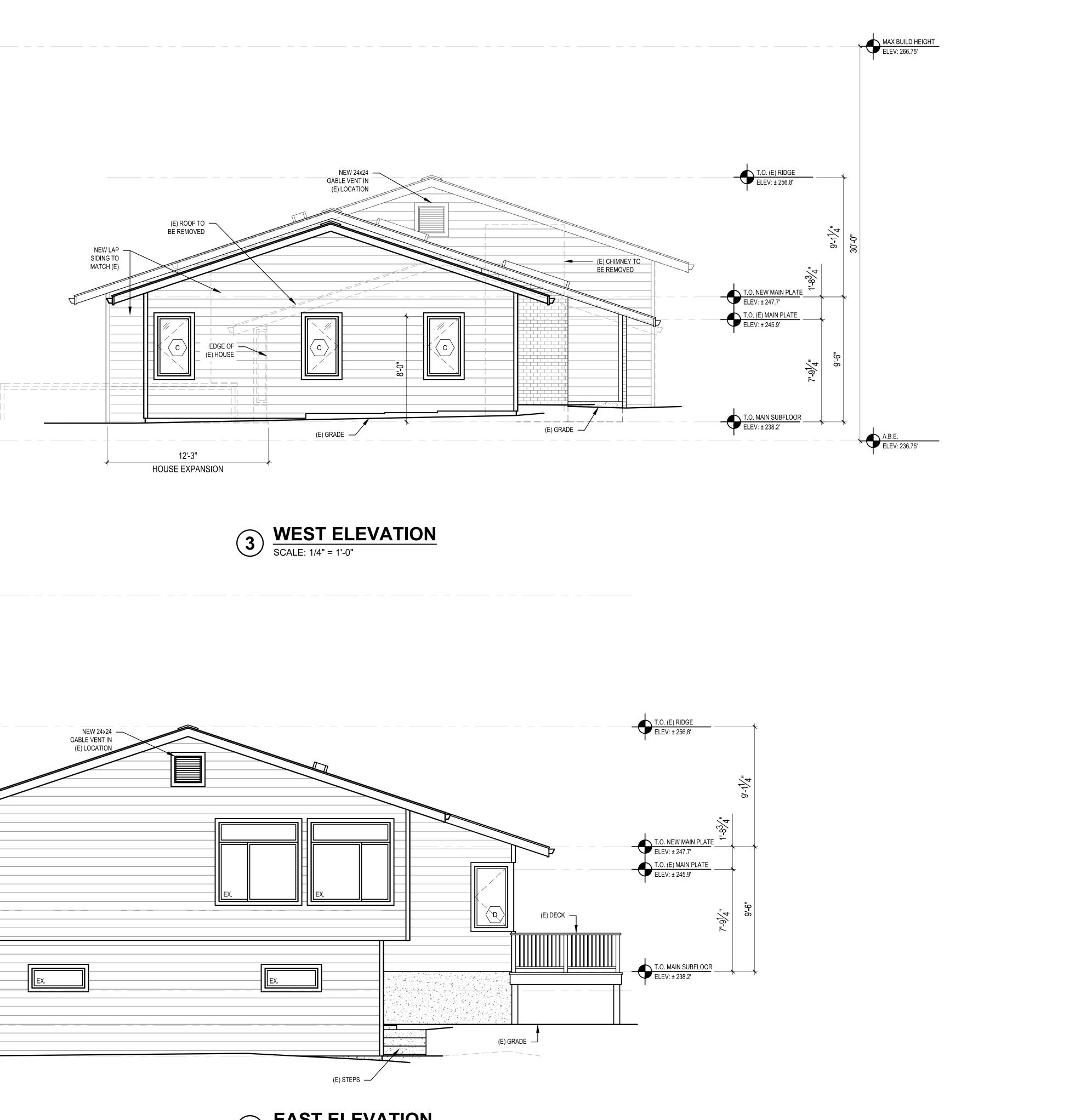
WINDOW	DESCRIPTION	WINDO	W SIZE	TEMP.	QTY.	TOTAL AREA	U-VALUE	NFRC	GLAZING	REMARKS & NOTES
MARK		WIDTH	HEIGHT			(SF)	(MIN.)	CERT.		
А	CASEMENT	2'- 10"	4'- 8"	Y	2	26.4'	.28	Y	LOW E / CLEAR	-
В	FIXED	11'- 7 1/2"	4'- 8"	Y	1	48.2'	.28	Y	LOW E / CLEAR	-
С	CASEMENT	2'- 6"	4'- 6"	Y	3	33.8'	.28	Y	LOW E / CLEAR	TEMPERED IN 1 LOCATION
D	CASEMENT	2'- 6"	4'- 8"	Y	1	11.7'	.28	Y	LOW E / CLEAR	-
E	SKYLIGHT	2'- 6"	4'- 0"	Y	2	20.0'	.28	Y	LOW E / CLEAR	-
F	SKYLIGHT	4'- 0"	5'- 0"	Y	2	40.0'	.28	Y	LOW E / CLEAR	-
G	SKYLIGHT	10'- 0"	1'- 0"	Y	1	10.0'	.28	Y	LOW E / CLEAR	-

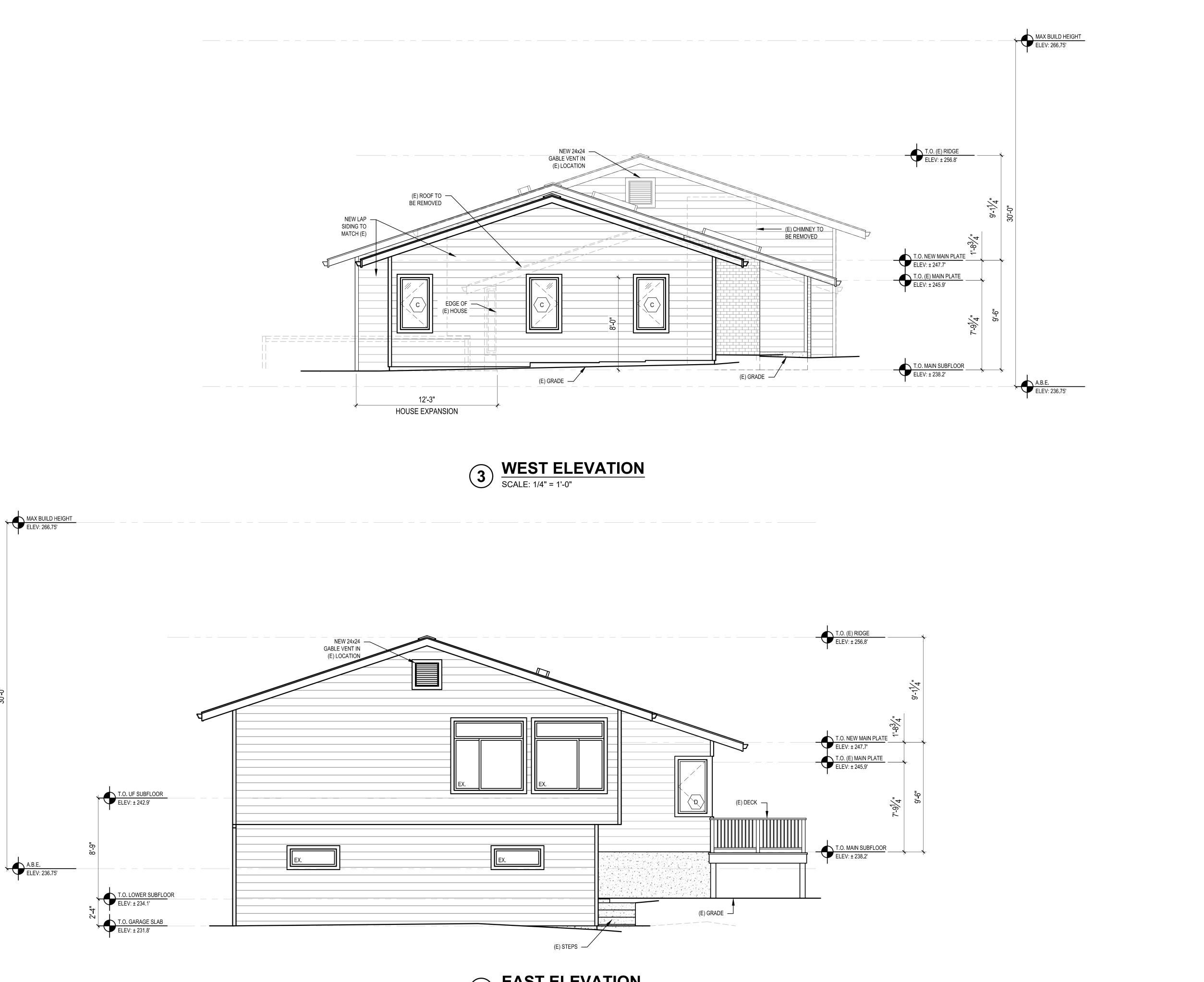


1'-0"

	REGISTERED ARCATTECT ARCATTECT BRADLEY J. STURMAN STATE OF WASHINGTON TEL: 425.451.7003 9- 103rd Ave NE Suite 203 Bellevue, WA 98004
	www.sturmanarchitects.com All Rights Reserved © 2021
UPPER ROOF ABOVE OUTLINE (E) EAVE VENT (V.I.F.): 37.0 LF	KNOTZ REMODEL 6020 94TH AVE SE MERCER ISLAND, WA 98040
ROOF A EXISTING	CALCULATION OW SCHEDULE
(E) 4:12 SLOPE (V.I.F.)	ROOF PLAN ROOF VENT DOOR/WIND
(E) EAVE VENT (V.I.F.): 37.5 LF	SNOISNAIL PLOT DATE: 4/12/2023 DRAWN BY: JM CHECKED BY: BJS
SCALE: IF SHEET IS LESS THAN 24" x 36", IT IS A REDUCED PRINT, REDUCE SCALE ACCORDINGLY SCHEMATIC SET 7/26/2021	SHEET A2.2





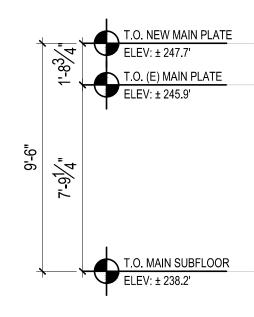


4 EAST ELEVATION SCALE: 1/4" = 1'-0"

DR DR:		All ©			
	EXTERIOR ELEVATIONS	Rights Rese 2021	5562 REGISTERED ARCHITECT		R M A N
<u>ج</u>	1		BRADLEY J. STURMAN		
		MERCER ISLAND, WA 98040)	TEL: 425.451.7003	9– 103rd Ave NE Suite 203 Bellevue, WA 98004

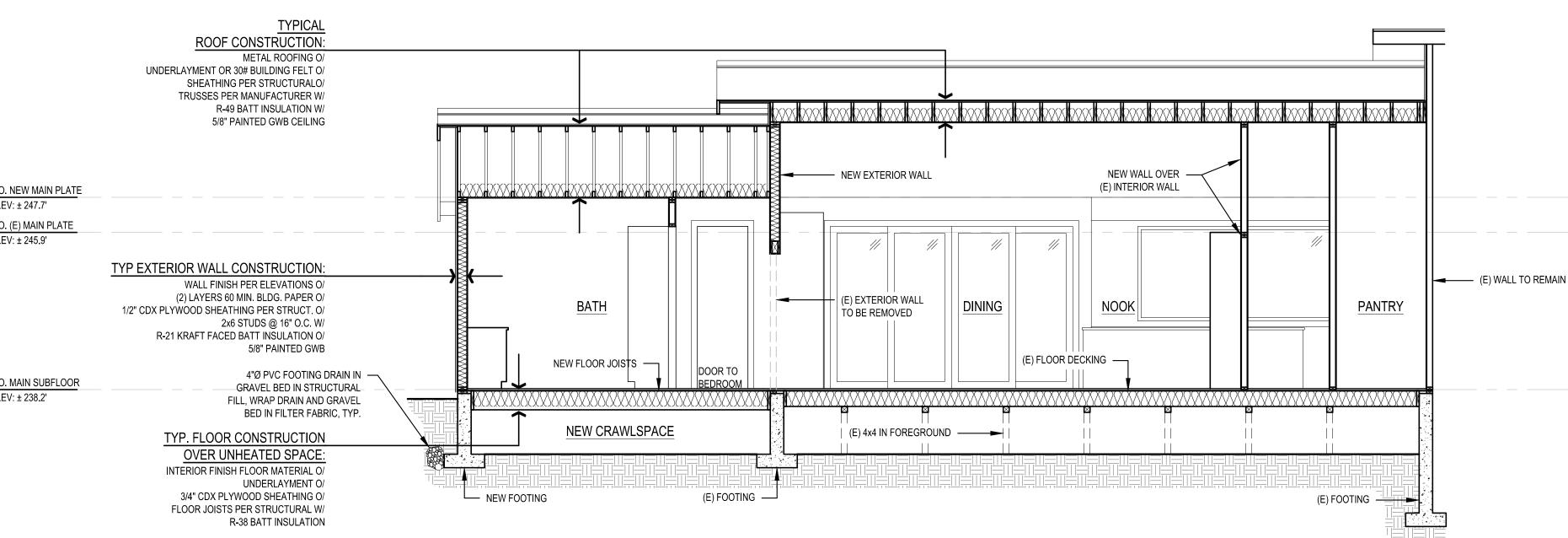
SCALE: IF SHEET IS LESS THAN 24" x 36", IT IS A REDUCED PRINT, REDUCE SCALE ACCORDINGLY PERMIT SET 4/12/2023

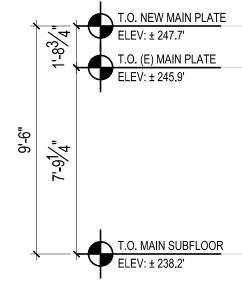
TYPICAL ROOF CONSTRUCTION: METAL ROOFING O/ UNDERLAYMENT OR 30# BUILDING FELT O/ SHEATHING PER STRUCTURALO/ TRUSSES PER MANUFACTURER W/ R-49 BATT INSULATION W/ 5/8" PAINTED GWB CEILING

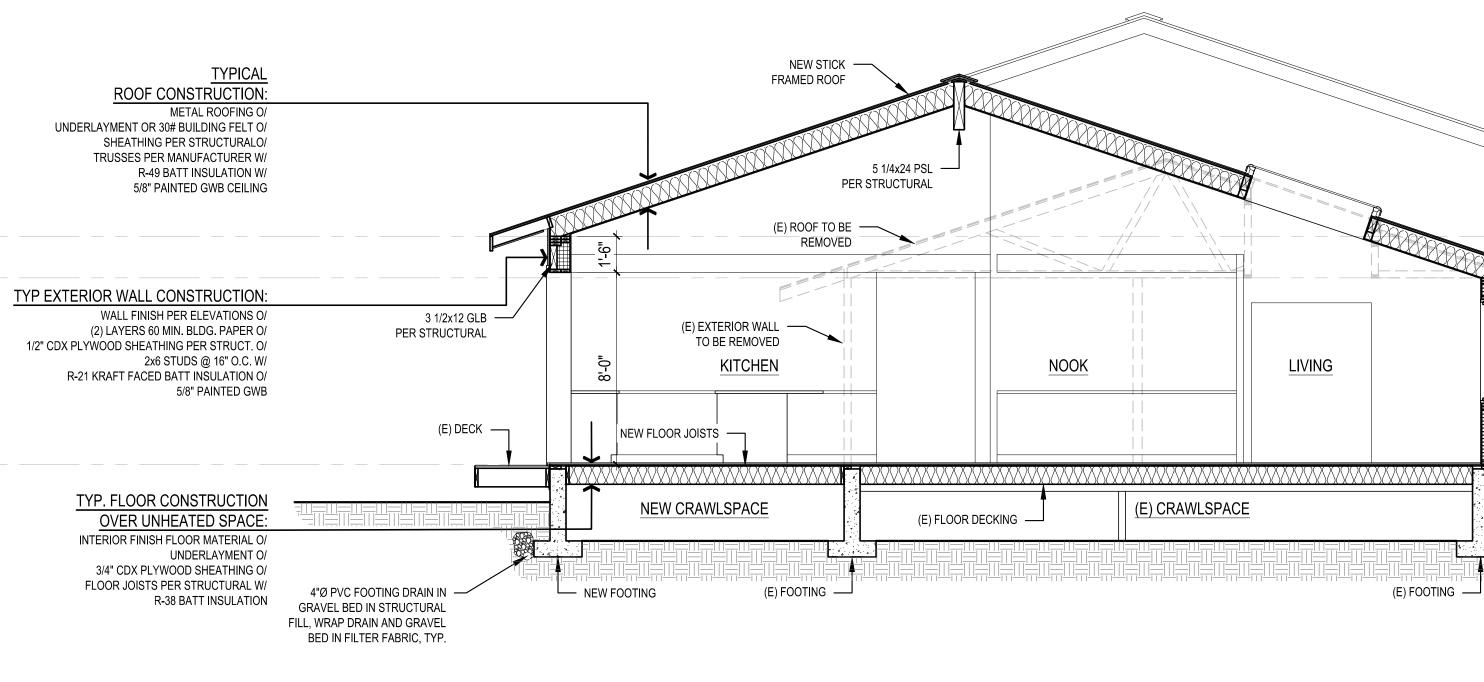


TYP. FLOOR CONSTRUCTION OVER UNHEATED SPACE

INTERIOR FINISH FLOOR MATERIAL O/ UNDERLAYMENT O/ 3/4" CDX PLYWOOD SHEATHING O/ FLOOR JOISTS PER STRUCTURAL W/ **R-38 BATT INSULATION**



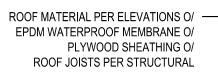








		WWW.sturmanan All Rights Rese	TEL: 425.451.7003 9– 103rd Ave NE Suite 203 Bellevue, WA 98004
(E) WINDOW TO REMAIN		KNOTZ REMODEL	6020 94TH AVE SE MERCER ISLAND, WA 98040
		ROISINER SNOISINER PLOT DATE: DRAWN BY:	 4/12/2023 JM
	SCALE: IF SHEET IS LESS THAN 24" x 36", IT IS A REDUCED PRINT, REDUCE SCALE ACCORDINGLY PERMIT SET 4/12/2023	SHEET	^{BJS}





WOOD FRAMED EXTERIOR WALLS -SIDING PER ELEVATIONS O/ (2) LAYERS -60 MIN. BLDG. PAPER

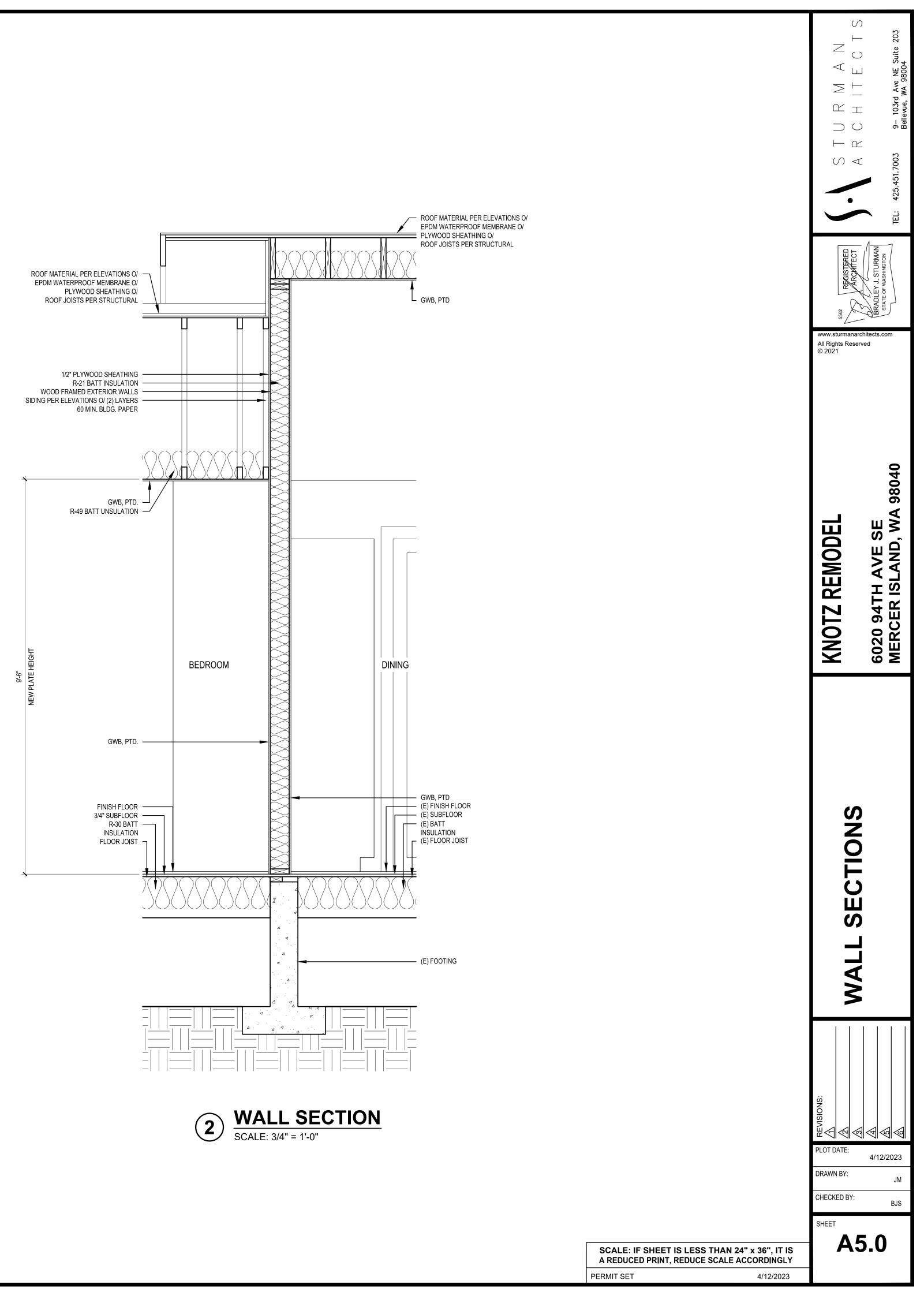


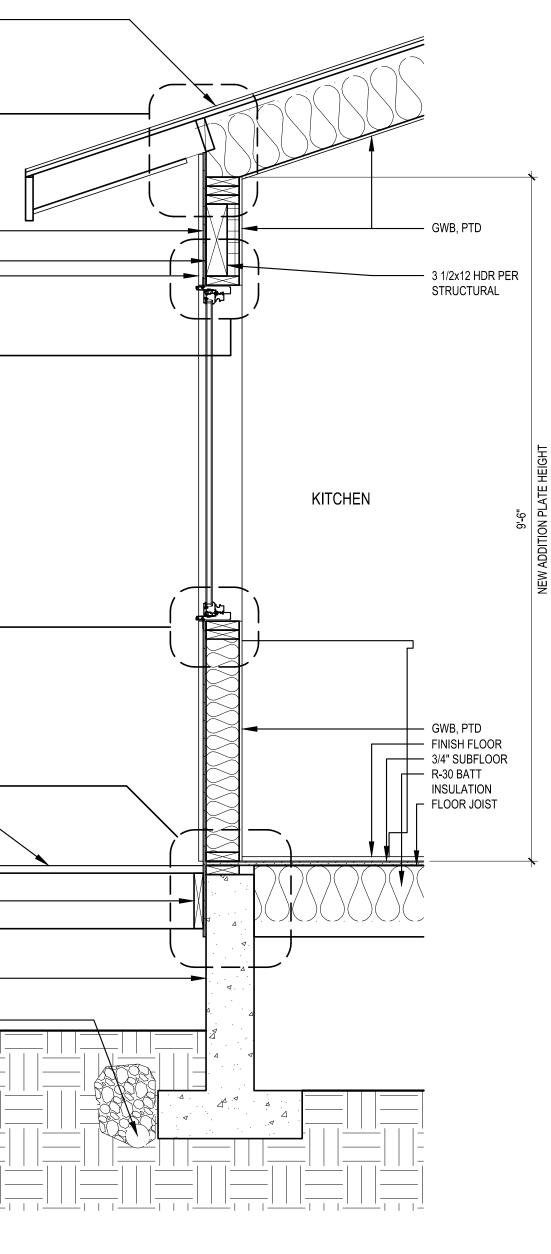


4 A6.0 DECKING — 2X DECK JOIST 2X LEDGER –

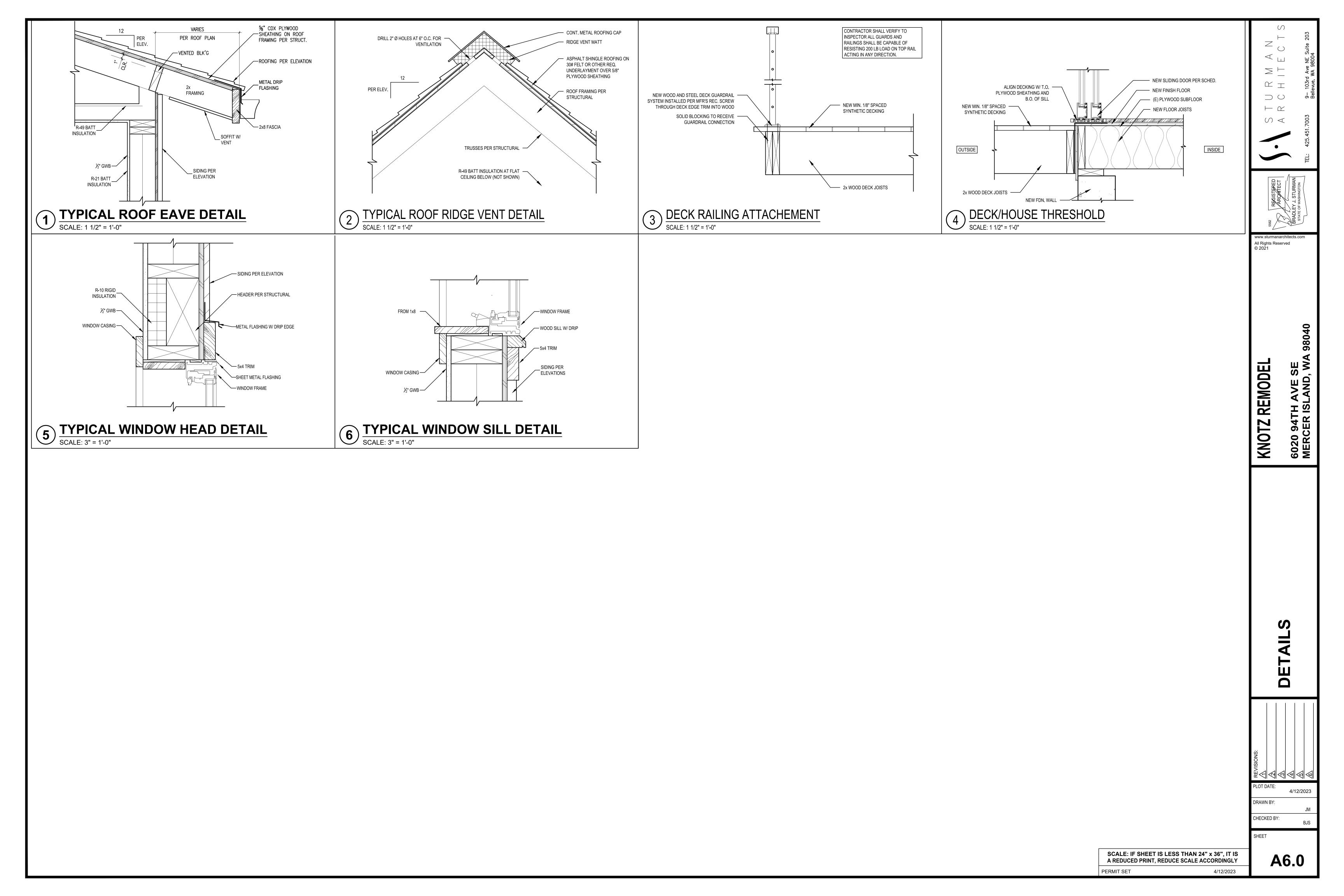
NEW FOOTING

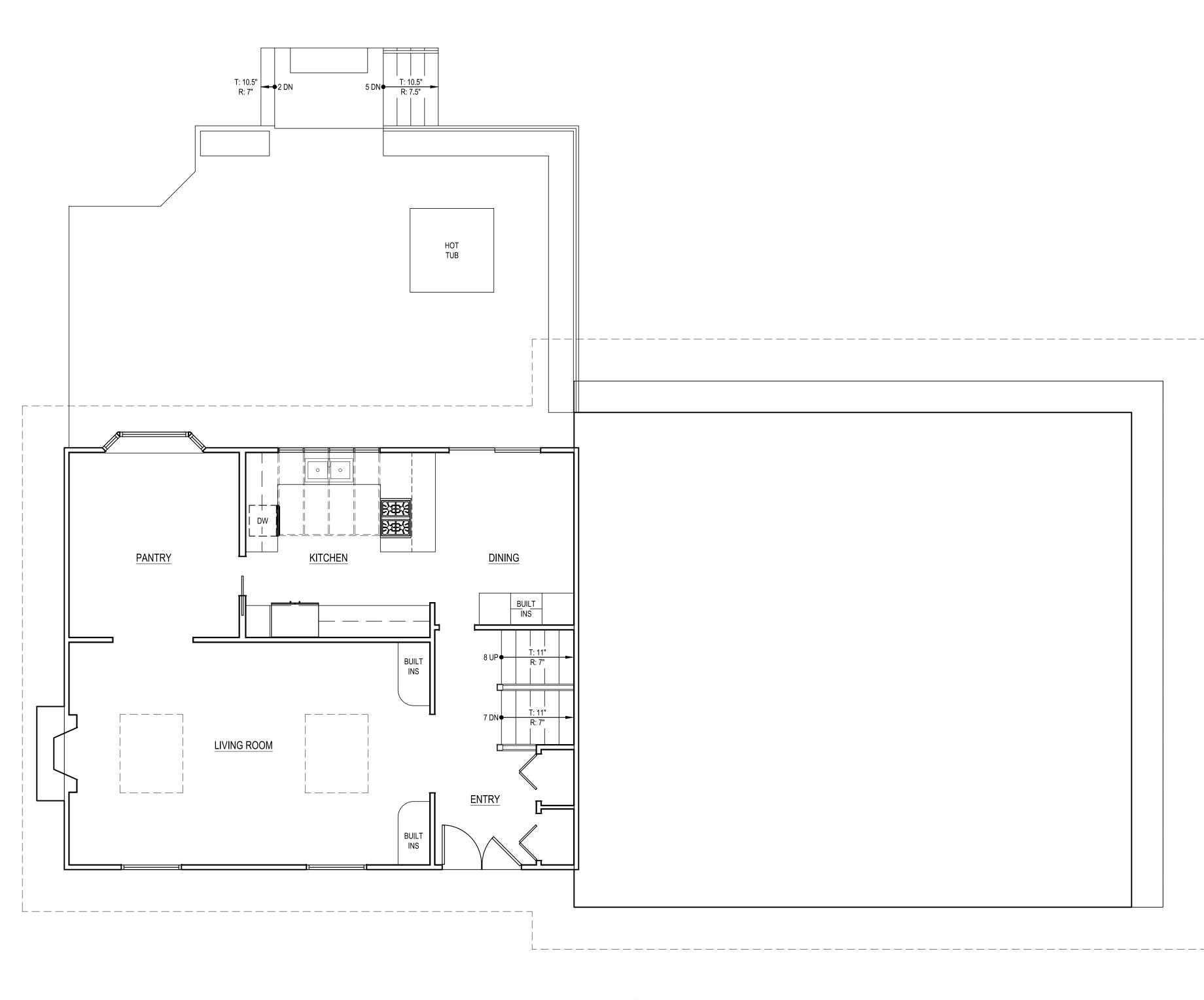
4"Ø PVC FOOTING -DRAIN IN GRAVEL BED, WRAP DRAIN AND GRAVEL BED IN FILTER FABRIC. TIGHTLINE TO (E) STORMWATER SYSTEM, TYP.

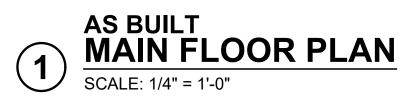






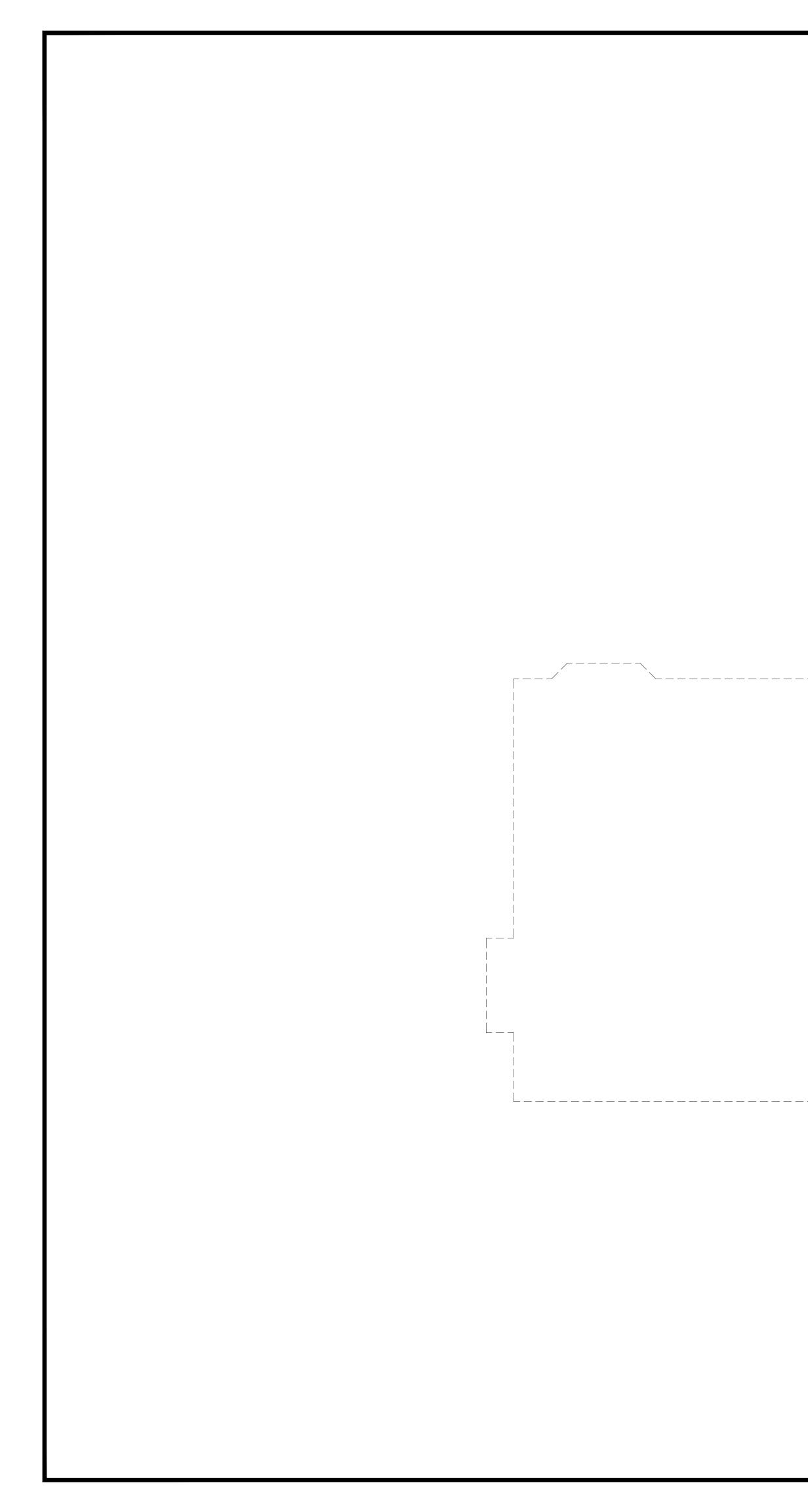


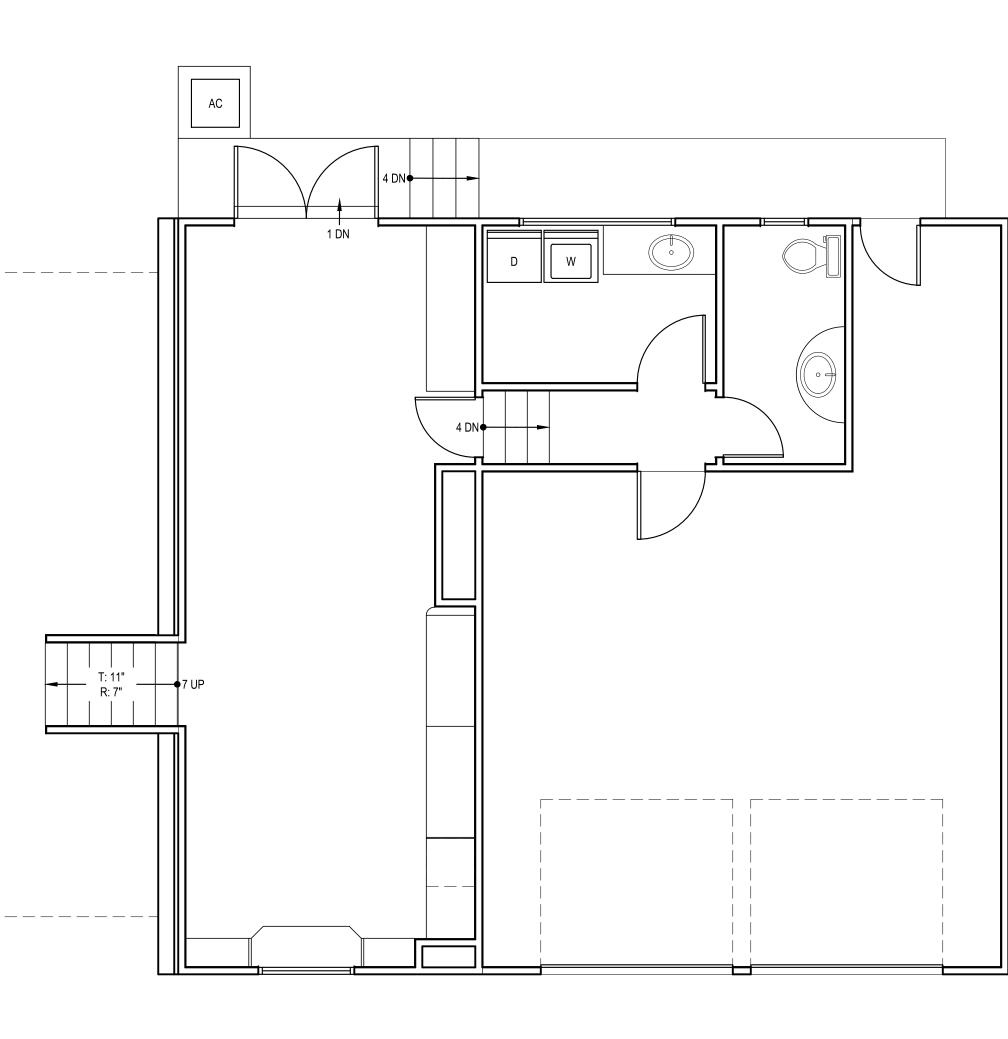




SHEET	DRAWN CHECK	D TOL9	AS BUILT		www.st		
AE			- MAIN FLOOR PLAN				СНІТЕСТS
31		4/12/2			Pitte of Washington		
	JM BJS	71 71		MERCER ISLAND, WA 98040))))	TEL: 425.451.7003	9- 103rd Ave NE Suite 203 Bellevue, WA 98004

	SS THAN 24" x 36", IT IS ICE SCALE ACCORDINGLY
PERMIT SET	4/12/2023







S 					Ŵ		
HEET	RAWN B		AS BUILT		2995 www.stur		M A N
AE			LOWER FLOOR PLAN	manarch : Reserve		A C	C H I T E C T S
32		4/12/			BRADLEY J. STURMAN		
	JM BJS	₹ 2023		MERCER ISLAND, WA 98040)	TEL: 425.451.7003	9– 103rd Ave NE Suite 203 Bellevue, WA 98004

	LESS THAN 24" x 36", IT IS EDUCE SCALE ACCORDINGLY
PERMIT SET	4/12/2023

GENERAL NOTES

- 1.1 Construction shall conform to the 2018 INTERNATIONAL RESIDENTIAL CODE and all other requirements of authorities having jurisdiction.
- 1.2 These drawings are the property of O.G. Engineering, PLLC ("Engineer"). These drawings and the information contained herein shall not be used for completion of or revisions to this project by others, extensions of this project or any other project without Engineer's express written permission.
- 1.3 Refer to Architectural Plans for all dimensions and elevations not shown. Do not scale drawings. The contractor shall verify all pertinent dimensions and existing conditions prior to beginning construction. Conflicts. differences in information, and omissions in drawings shall be brought to the attention of the Engineer for resolution prior to construction. Changes from the drawings shall be made only with the prior approval of the Engineer. All work is subject to review and approval by the local building department. All work shall conform to all permit and building department requirements. All details shall be considered typical at similar conditions. Details shall be used where applicable, unless otherwise noted. Details intend to show concepts that may not exactly match specific site conditions. All work shown on these drawinas is new unless noted as existing.
- 1.4 The contractor shall be solely responsible for jobsite and construction safety and compliance with all current safety regulations. Jobsite visits performed by the Engineer do not include a review of the adequacy of the contractor's safety measures. The Engineer has no authority to exercise any control over any construction contractor or their employees in connection with their work or any health or safety precautions. Only the final, permanent structure is shown on these drawings. The contractor shall be solely responsible for the means and methods of construction, including but not limited to construction sequencing and providing all necessary shoring, bracing and other temporary supports during construction. The contractor shall be solely responsible for obtaining all necessary independent engineering reviews of all temporary conditions and support systems during construction.
- 1.5 Utility information is not shown on these drawings. The contractor shall be solely responsible for locating and protecting utilities prior to and during construction. The contractor shall be solely responsible for all damage to utilities resulting from their work, and all damage to utilities shall be repaired solely at the contractor's expense.
- 1.6 All waterproofing and drainage information shown on these drawings is for illustrative purposes only. Waterproofing and drainage are the design responsibility of others.

2.0 DESIGN BASIS - BUILDING STRUCTURES

2.1	Vertical Loads (psf) Roof	Dead 18*	Live	Snow 25
	Main Floor	10	40	
	Deck	10	60	
	*Includes 4psf for se	olar—ready zones		

- 2.2 Seismic Design Data (per the 2018 IBC) Risk Category: II Importance Factor: le=1.0 Site Coordinates: 47.5493°N, 122.2138°W Mapped Spectral Response Acceleration: Ss=1.45, S1=0.50 Site Class: Default D Spectral Response Coefficients: Sds=1.16 Seismic Design Category: D Main Seismic Force-Resisting System: Wood Structural Panel Shear Walls Response Modification Factor: R=6.5 Seismic Response Coefficient: Cs=0.18 Redundancy Factor: $\rho = 1.3$ Over-strength Factor: $\Omega=2.5$ Analysis Procedure Used: Equivalent Lateral Force Procedure
- 2.3 Wind Design Data (per the 2018 IBC) Risk Category: II Basic Wind Speed: 97 mph Exposure Category: C Topographic Factor: 1.00 (Per Mercer Island Wind Load Map)

3.0 INSPECTIONS

The construction work shall be inspected as required by the SRC Section R106. The contractor is solely responsible for understanding the requirements of and coordinating all inspections, observations and testing and ensuring that all work is performed to the satisfaction of the inspector.

4.0 FOUNDATIONS

4.1 The following foundation & retaining wall design criteria are assumed, have not been verified by a geotechnical engineer and therefore must be approved by the building official. If design criteria are found to be different than assumed, notify Engineer for additional requirements prior to construction:

*Allowable Vertical Bearing Pressure:

4.2 Footing & Slab on Grade Excavations

Remove any deleterious, loose or softened material from footina & slab on grade excavations and compact sub-grades to a firm and unyielding condition. If loose sub-grades can not be adequately compacted, over-excavate loose material to competent soil and replace with properly compacted structural fill. Do not allow water to stand in excavations; if sub-grades become softened before concrete is cast, excavate softened material and replace with properly compacted structural fill at no additional cost to the owner. Structural fill and compaction requirements are the design responsibility of others.

2000 psf

- 5.0 MATERIALS
- 5.1 Wood:

5.1.1 All 2x & 3x sawn lumber shall be Hem Fir grade number 2. and all 4x and larger lumber shall be Doug Fir grade number 1, U.O.N. Mudsills and all sawn lumber in contact with concrete. masonry, ground, exposed to weather or moisture, shall be P.T. Preservative retention levels in P.T. wood shall meet the requirements of the applicable use category in accordance with AWPA U1-16, and shall not exceed those required to comply with AWPA Use Category UC4A. Do not use wood treated with ACZA. Field-cut ends, notches and drilled holes of P.T. wood shall be treated in the field in accordance with AWPA M4. P.T. is not required at naturally decay-resistant (i.e. redwood, cedar etc.) sawn lumber members.

5.1.2 Engineered Wood Framing Members shall be TrusJoist® or approved equal. 'PSL' denotes Parallam 2.2E for beams and 1.8E for posts. 'LSL' denotes Timberstrand 1.55E for members with depth equal to or greater than $9\frac{1}{2}$ ", and 1.3E for members with depth less than $9\frac{1}{2}$ ". 'LVL' denotes Microllam 2.0E.

5.1.3 Glulam framing members shall be DF/DF, stress class 24F-1.8E, combination symbol 24F-V8, U.O.N. Glulam framing members exposed to weather shall be treated with HI-CLEAR II wood preservative or approved equal. Field-cut ends, notches and drilled holes of treated alulam framing shall be re-treated in the field in accordance with AWPA M4. Surfaces, ends, notches and drilled holes in alulam framing exposed to weather shall be sealed in accordance with the recommendations of the manufacturer, APA and AITC after preservative treatment.

5.1.4 All wood framing members shall have 19% maximum moisture content at time of installation.

5.2 Concrete:

Hardrock, normal-weight concrete with a minimum 28-day compressive strength of 3,000 psi for concrete exposed to weather and 2,500psi for concrete not exposed to weather. Slump range shall be 3-5 inches. Maximum aggregate size shall be 1". Maximum water/cement ratio shall be 0.5. Concrete exposed to weather shall be air-entrained with total air content between 5%-7% of total concrete volume.

5.3 Reinforcing Steel Bars:

- ASTM A615, Grade 60
- 5.4 Post-Installed Dowels & Anchors into Existing Concrete & CMU
- Epoxy: Simpson SET-3G (Installed & inspected per ICC No. ESR-4057)

5.5 Bolts and Threaded Rods:

5.5.1 Threaded Rod: ASTM F1554 Grade 36

5.5.2 Sill Anchor Bolts: ASTM A307

Bent bar "J" anchor bolts shall have a hook with a 90-dearee bend with an inside diameter of three bolt diameters, plus an extension of one and one half bolt diameters at the free end.

5.5.3 Bolts in Timber Connections: ASTM A307

- 5.5.4 Bolts in Steel Connections: ASTM A325-N (High-Strength)
- 5.6 Structural Steel:

Wide Flange (W):	A992 (Fy = 50 ksi)
Rectangular Tube (HSS): Plate and Bar:	A500 Gr. B (Fy = 46 ksi) A36 (Fy = 36 ksi)
	A30 (1y = 30 ks)

- 6.0 CONCRETE CONSTRUCTION
- 6.1 Concrete elements shall be constructed in single continuous pours, without construction joints, unless otherwise approved by the Engineer. Reinforcement shall be the longest lengths practical. Splices in rebar are not allowed in footings or walls less than 20 feet long. Lap splices shall be staggered at least 2 ft. in adjacent bars. Where reinforcement or anchor edge distances are noted on the drawings as "clear", the distance shall be taken from the face of reinforcement or anchor to edge of concrete. Cast-in-place reinforcement and anchor bolts shall be installed prior to concrete placement and <u>shall not</u> be "wet-set" into freshly poured concrete.
- 6.2 Reinforcement installation details, including rebar bends, hooks, splices and development lengths shall be in accordance with the requirements of IRC Section R608.5.4, U.O.N. Concrete materials. forms, mixing and delivery shall be in accordance with the requirements of the IRC Section R404.1.3.3.
- 6.3 Concrete Coverage over Reinforcing Steel
- Unless otherwise noted, maintain the minimum concrete cover to face of reinforcement or anchors as follows:
- 1) 3" Where concrete is cast against and permanently exposed to
- earth except slab on grade. 2) 2" Where concrete is exposed to earth but formed, or exposed
- to weather.
- 3) $1\frac{1}{2}$ Where concrete is not exposed to earth or weather.

7.0 WOOD CONSTRUCTION

7.1 General Framing

Connections not specified on these drawings shall conform to the IRC fastening schedule, refer to Table R602.3(1). Depth of all posts in walls shall match stud depth, U.O.N. Block floor joist space solid under posts and cripple studs supporting headers and continue support to foundation. Face nail all plies of multi-ply studs with 10d@6"o.c. Obtain approval from engineer prior to ripping or creating notches or holes in framing members, U.O.N. Install double joists below all new interior walls parallel to floor joists and solid blocking below all new interior walls perpendicular to floor joists (NSFC on plan), U.O.N. All beams shall be continuous across supports unless explicitly shown as multiple pieces. Install full depth blocking between framing members over supports, unless otherwise noted. Install 2x4 blkg btwn adjacent framing members @24"o.c. over interior partitions. All flush beams framina into walls shall continue to back edge of supporting dbl top plate; stop rim joist each side of beam where occurs.

7.2 Engineered Wood Framing

See TrusJoist "Installation Guide for Floor and Roof Framina" (TJ-9001) for allowable holes in engineered wood beams. Grade stamp info must be maintained on ripped engineered wood members: refer to TrusJoist Technical Bulletin TB-305 for requirements pertaining to re-sawn engineered wood.

7.3 Fasteners

Nails specified on these drawings are common nails, U.O.N. Fasteners in contact with P.T. wood, exposed to weather or in contact with ground shall be hot-dipped galvanized per SRC Section 317.3, or shall have equivalent corrosion resistance. Dissimilar metals & coatings shall not be in contact. Bolt holes shall be a minimum of $\frac{1}{32}$ " to a maximum of $\frac{1}{16}$ " larger than the bolt diameter. Bolts shall not be forcibly driven, and shall be tightened to the snug-tight condition. Install standard cut washers under all bolt heads and nuts bearing against wood.

7.4 Connectors

Connectors specified on these drawings are manufactured by the SIMPSON STRONG-TIE® Company. Refer to latest catalog for information not specifically noted herein. Connectors in contact with P.T. wood, exposed to weather or in contact with ground shall be ZMAX or HDG galvanized. All connectors shall receive the maximum number of fasteners, U.O.N. Dissimilar metals & coatings shall not be in contact. Shim gaps in connectors for different framing sizes with plywood as required. Non-field-adjustable hangers specified as sloped or skewed shall be manufactured sloped or skewed.

7.5 Wood Structural Panels

WSPs shall bear the APA trademark and shall meet the requirements of the latest edition of USDOC PS1 or PS2. Use 10d common wire nails to fasten panels with $1\frac{1}{2}$ " minimum penetration into framing at all panel edge and field nailing, U.O.N. Nails shall be located at least $\frac{3}{8}$ " from panel ends and edges. Stagger nails at adjoining panel edges. Drive nail heads flush with panel surface. Maintain $\frac{1}{8}$ " gap between all adjoining panel edges. Center interior panel joints on framing members or blocking. Provide $\frac{1}{2}$ " space between untreated panel and concrete or masonry. Minimum panel dimension shall be 2'-0". Panel storage and handling during transport and construction shall be in accordance with APA recommendations and shall protect the panels from prolonged exposure to moisture from rain, snow, ground or other sources. WSPs permanently exposed to weather shall be exterior grade.

7.6 Shear Walls and Exterior Wall Sheathing

7.6.1 Shear walls are noted on the plans. Shear walls shall be sheathed with $\frac{1}{2}$ " APA RATED SHEATHING, EXPOSURE 1 WSPs with a span rating of $\frac{32}{16}$, U.O.N. Panels shall not be less than 4'-0" x8'-0'', except at boundaries and changes in framing. Panels shall be laid with strength axis vertical. Install 2x blkg under all unsupported panel edges; all panel edges shall be supported by and fastened to min. 2x common studs or blocking. U.O.N. on shear wall schedule. Edge nail panels to posts within shear walls. Install double stud or min. 4x post at the ends of all shear walls. Provide solid blocking under double studs & posts between floors and continue support to foundation. See shear wall schedule for more information.

7.6.2 WSP Wall Nailing, U.O.N.:

Panel Edge Nailing: 10d@6"o.c. maximum. Intermediate (Field) Nailing: 10d@12"o.c. maximum.

7.6.3 All new exterior walls not called out as shear walls shall be sheathed on their exterior face with $\frac{1}{2}$ " APA RATED SHEATHING, EXPOSURE 1 WSPs with a span rating of $\frac{32}{16}$ and nailing per note 7.6.2., U.O.N. All other fasteners & requirements shall conform to the shear wall schedule for wall type (1).

7.7 Holdowns and Tiedown Straps

Holdowns and tiedown straps shall be attached to double studs or min. 4x posts, U.O.N. See latest Simpson Catalog for additional requirements not noted herein. See holdown schedule for anchor bolt sizes and additional specifications. Refer to note 7.1 for nailing and framing requirements at holdown/tiedown posts. Install solid post at shear wall corners or intersections where holdowns/tiedowns occur. All holdowns/tiedowns shall have the maximum number of fasteners.

7.8 Sill Anchor Bolts

There shall be a minimum of two sill anchor bolts per piece with one bolt located not more than 12" or less than $4\frac{1}{2}$ " from each end of each piece. Holes in sills for bolts shall not be oversized. Sill anchor bolts shall be $\frac{5}{2}$ with 7" min. embed. into concrete. Sill anchor bolts into existing concrete shall be all-thread rod, drill and epoxy. See shear wall schedule for spacing of sill anchor bolts in shear walls. Maximum sill anchor bolt spacing at non-shear-walls shall be 6'-0" o.c. at interior walls and 4'-0" o.c. at exterior walls. All sill anchor bolts at shear walls and mudsills shall be installed with 0.229"x3"x3" steel plate washers. Edge of sill anchor bolt plate washers shall be located $\frac{1}{2}$ " max. from inside face of wall sheathing or rim joist where occurs.

7.9 Floor and Roof Sheathing

7.9.1 Wood structural panel sheets at floors and roofs shall be laid with strength axis perpendicular to supports and continuous over two or more spans, unless otherwise noted on drawings. Stagger adjacent panels 4'-0"o.c. lengthwise.

7.9.2 Unless otherwise noted, typical roof sheathing shall be unblocked §" APA RATED SHEATHING, EXPOSURE 1 WSPs with a span rating of $\frac{40}{20}$. Panels shall be fastened to framing members with 10d nails @6"o.c. at all supported panel edges and 10d nails @12"o.c. intermediate (field) nailing. Install 'PSCL' sheathing clips (one mid-way between each support) at all unsupported panel ioints.

7.9.3 Unless otherwise noted, typical floor sheathing shall be unblocked $\frac{3}{4}$ " APA RATED STURD-I-FLOOR EXPOSURE 1 WSPs with a span rating of $\frac{48}{24}$ and T&G edges. Panels shall be fastened to framing members with 10d nails @6"o.c. at all supported panel edges and 10d nails @12"o.c. field nailing. Glue sheathing to all supports (including blocking) with $\frac{1}{4}$ " minimum beads of approved adhesive meeting APA specification AFG-01.

7.10 Metal-Plate-Connected Wood Trusses

7.10.1 The design, manufacture and installation of trusses shall be in accordance with the requirements of ANSI/TPI 1 and the IRC Section R502.11.

7.10.2 Trusses, structural fascia, their connections to other trusses/fascias, and truss eave blocking are the design responsibility of the supplier, and shall be designed by a civil or structural engineer licensed in the State of Washington ("Truss Designer"). Trusses shall be designed to support the following specific unfactored loads in addition to their self-weight:

Vertical Roof Loads - Top Chord *Dead: 14 psf (Does not include truss self-weight) *Snow: 25 psf *Wind: -40 psf (uplift)

<u>Vertical Ceiling Loads – Bottom Chord</u> *Dead: 5 psf (Does not include truss self-weight)

*Live: 10 psf (Does not act concurrently with roof live load)

7.10.3 Trusses shall not rely on interior walls for support, U.O.N.; trusses shall be designed to span between exterior bearing walls.

7.10.4 Trusses shall be braced to provide lateral stability and prevent rotation in accordance with the SBCA BCSI "Guide to Good Practice for Handling, Installing and Bracing of Metal-Plate-Connected Wood Trusses". Bracing shall be designed and specified by the truss designer.

7.10.5 Trusses and their connections shall not be notched, cut. spliced or otherwise altered or damaged in any way without the prior written consent of both the E.O.R. and truss designer. 7.10.6 Truss design drawings and calculations, prepared by a civil or structural engineer licensed in the State of Washington in accordance with the SRC Section R502.11.4, shall be submitted to

the contractor, architect, engineer and local building official for review and acceptance prior to fabrication, and shall be provided with the shipment of trusses to the job site.

7.10.7 Attach top plates of interior, non-bearing partition walls to truss bottom chords with 'STC' clips, leaving a $\frac{1}{4}$ " to $\frac{1}{2}$ " vertical gap between bottom of truss and top of plate. Attach adjacent gypsum board ceiling to top plate with 'DS' clips. Do not fasten gypsum board ceiling to truss bottom chord within 16" of top plate.

ABBREVIATIONS

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ADJ.

ALT.

B.F.

BLKG

BLW.

BOTT.

C.I.P.

C.J.

CLR.

CONT.

CSK.

ø

DBL.

DF

DIM

D.J.

D.R.

E.J.

E.N.

EQ.

E/W

(E)

F.J.

F.N.

FTG

G.L.

GLB

G.C.

HDR

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PSF

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R.R.

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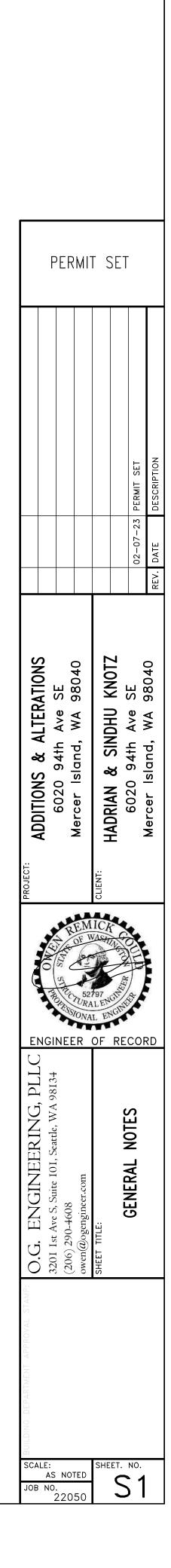
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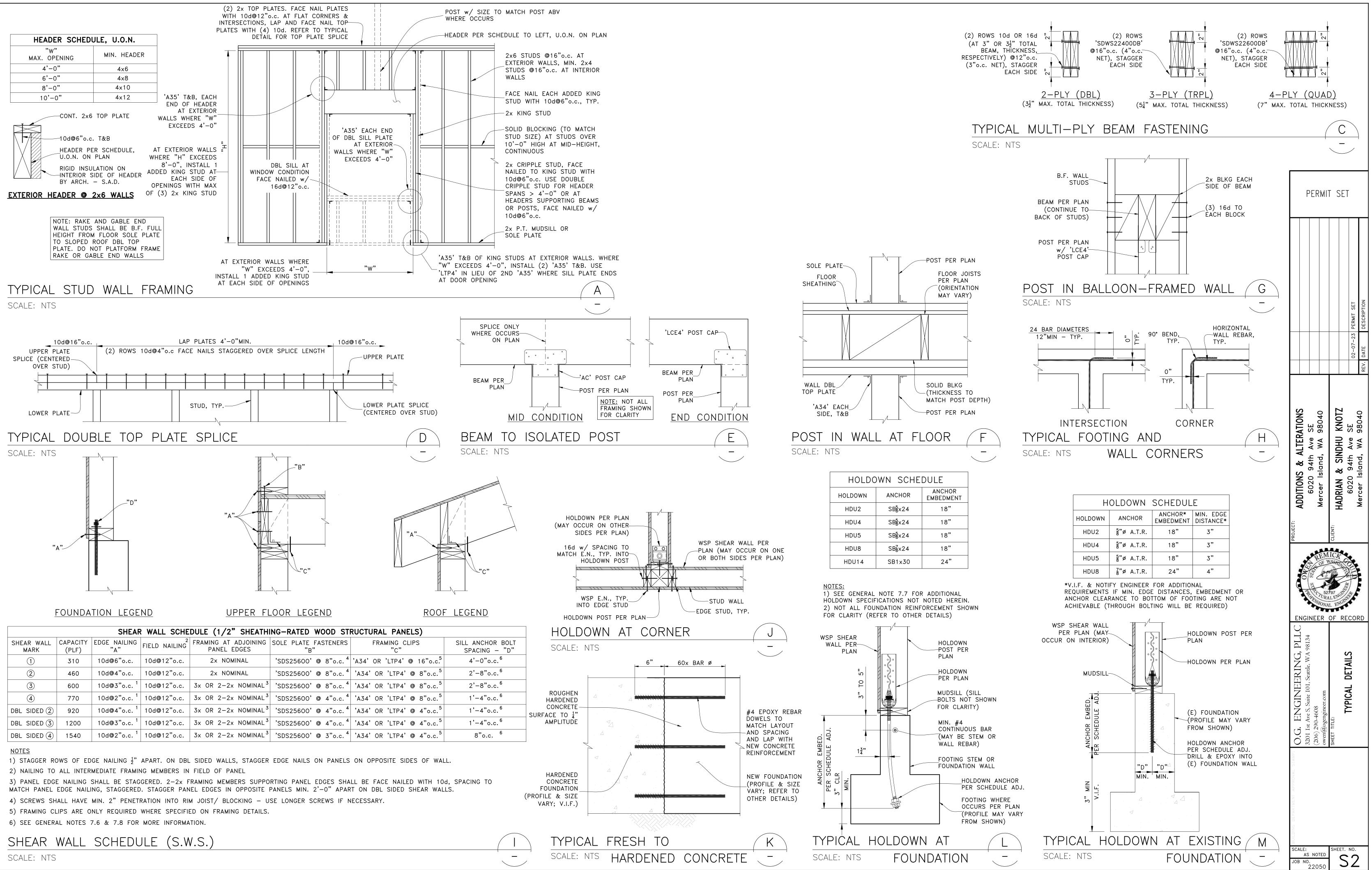
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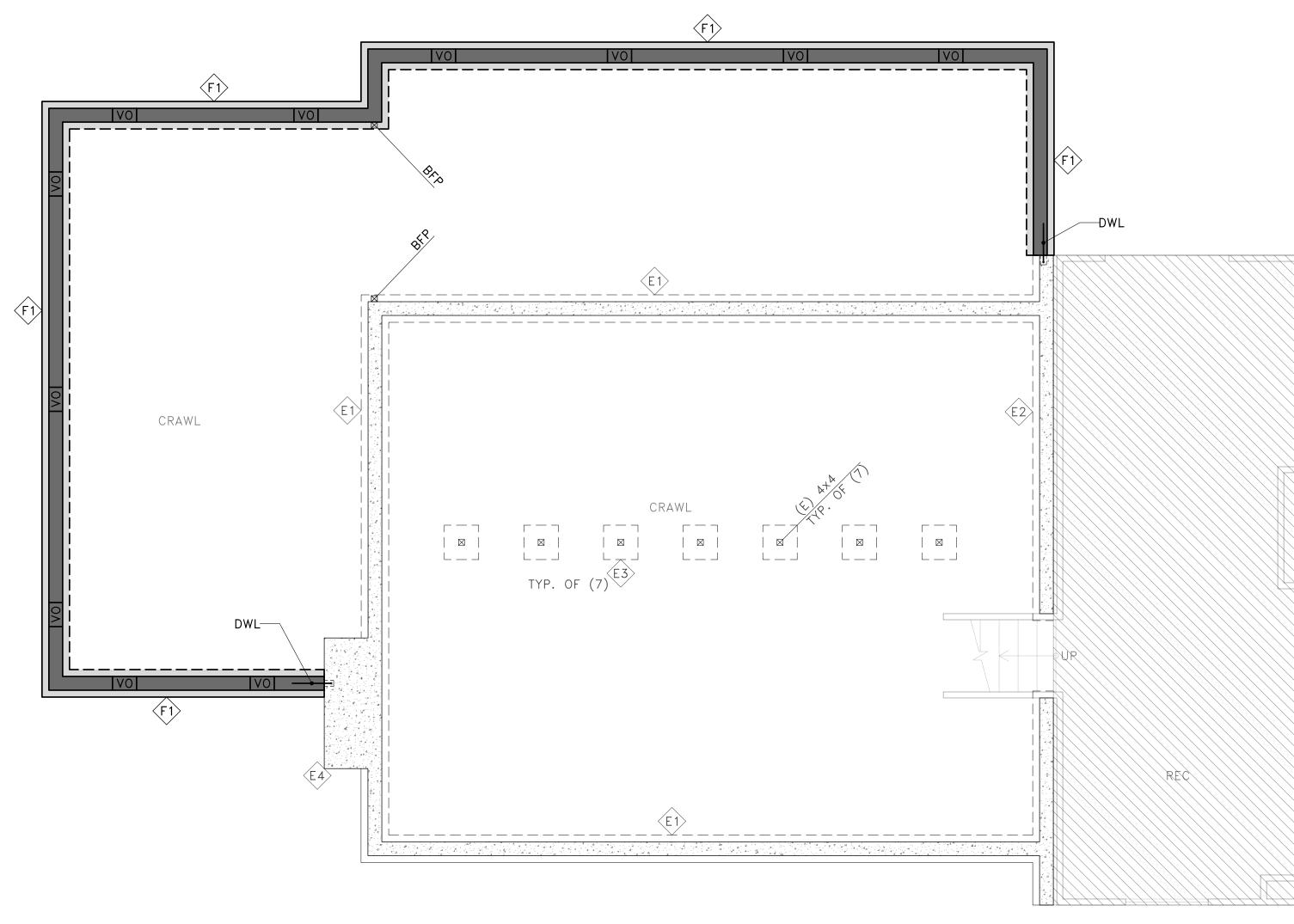
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^{1.0} GENERAL



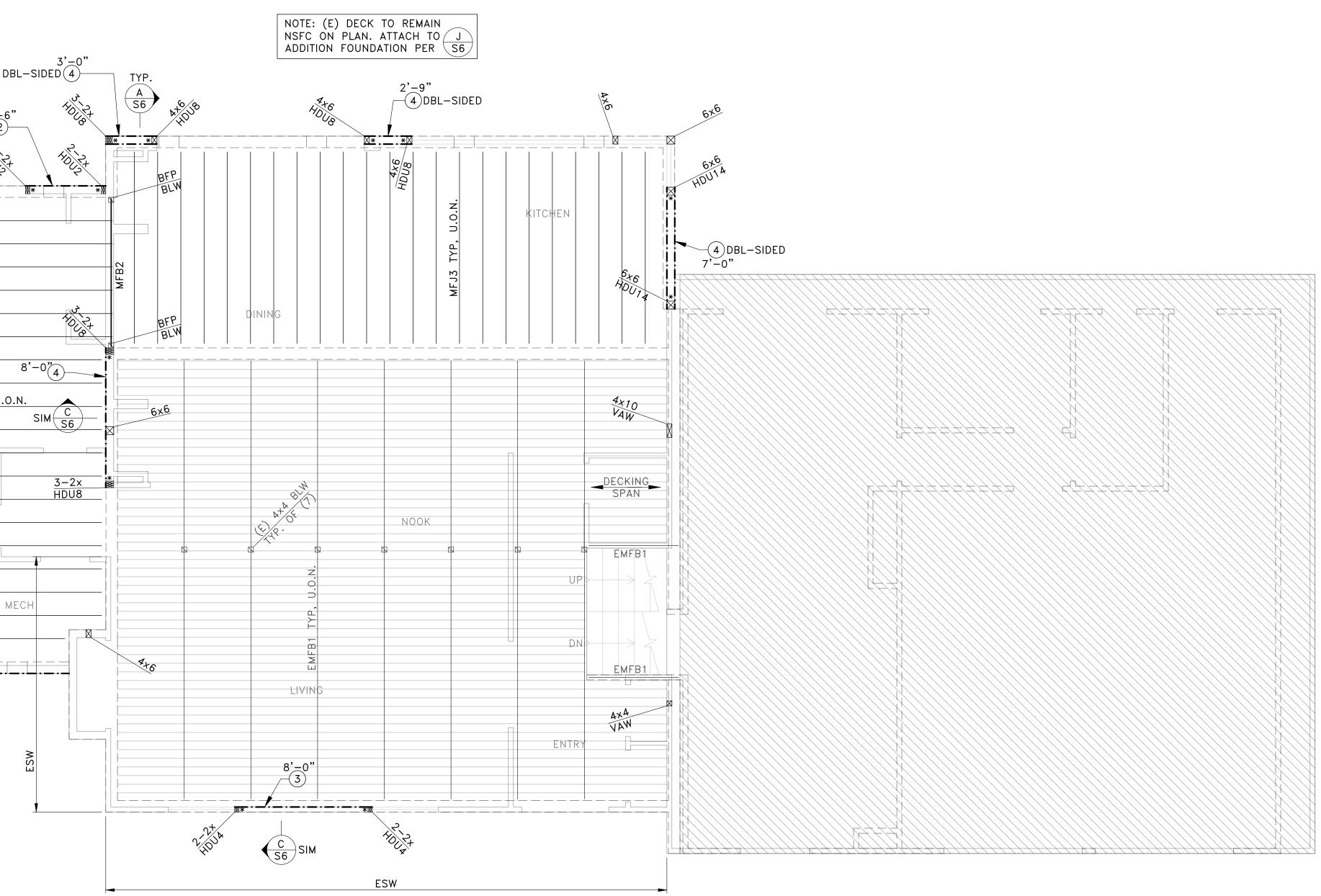
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E2>	WALL w/ 16" WIDE T-FOOTING (E) 8" BASEMENT FOUNDATION WALL w/ 16" WIDE T-FOOTING
E3	(E) 20"SQ. CRAWLSPACE PAD FOOTING
E4	(E) CONCRETE CHIMNEY PAD (V.I.F.)



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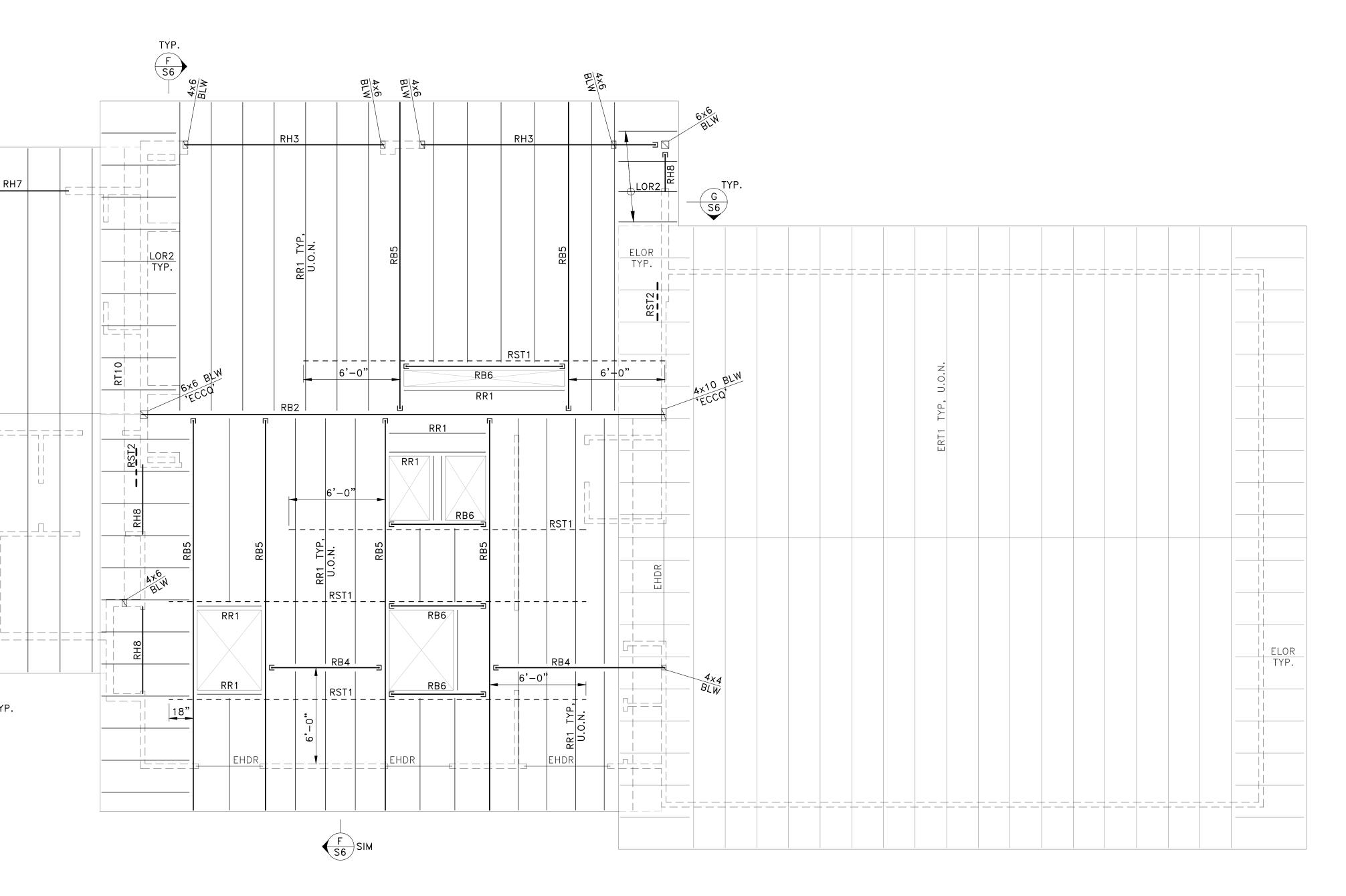
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where occurs (Post with to match BEAM, NOT EACH SIDE OF JOIST OR BEAM OVER SUPPORT FRAMING SCHEDULE ALLOUT JOIST/BEAM MFJ1 2x12 @16"o.c. JB212A (a) (b) (DROPPED) N/A MFJ3 2x10 @16"o.c. JB210A (a) (b) (b) (c) (c) (c) (c) (c) (c) (c) (c) (c) (c		T	JOIST OR BE	EAM BEARING ON		8'-0"1	BED 1
FRAMING SCHEDULE ALLOUT JOIST/BEAM HANGER (LUON, OH PLAN) REFER TO DETAIL(S) (OR SEE NOTES BLW) MFJ1 2x12 @16"o.c. JB212A A E S6/S6 MFJ3 2x10 @16"o.c. JB210A A E S6/S6 Image: Schedule in the second in the	5		WHERE OCCU SHOWN FOR	JRS (POST WIDT CLARITY). INSTA	H TO MATCH BEAM, NOT ALL FULL—DEPTH BLKG		
ALLOUT JOIST/BEAM HANGER (U.O.N. ON FLAN) REFER TO DETAIL(S) (or see Notes BLW) MFJ1 2x12 @16"o.c. JB212A A E SS S6 MFJ3 2x10 @16"o.c. JB210A A E SS S6 Image: See Notes BLW) Image: See Notes BLW) Image: See Notes BLW) Image: See Notes BLW Image: See Notes BLW) Image: See Notes BLW) Image: See Notes BLW Image: See Notes BLW) Image: See Notes BLW) Image: See Notes BLW Image: See Notes BLW) Image: See Notes BLW) Image: See Notes BLW Image: See Notes BLW) Image: See Notes BLW) Image: See Notes BLW Image: See Notes BLW) Image: See Notes BLW Image: See Notes BLW Image: See Notes BLW Image: See Notes BLW Image: See Notes BLW Image: See Notes BLW Image: See Notes BLW Image: See Notes BLW Image: See Notes BLW Image: See Notes BLW Image: See Notes BLW Image: See Notes BLW Image: See Notes BLW Image: See Notes BLW Image: See Notes BLW Image: See Notes BLW Image: See Notes BLW Image: See Notes BLW Image: See Notes BLW Image: See Notes BLW Image: See Notes BLW Image: See Notes B		FRA				TYP. S6	
MFJ1 2x12 @16"o.c. JB212A A E S6 S6 MFB2 4x10 (DROPPED) N/A N/A MFJ3 2x10 @16"o.c. JB210A A E S6 S6 Image: Second secon	CALLOUT			HANGER		2-2+2	MFJ1 TYP, U.
MFB2 4×10 (DROPPED) N/A N/A MFJ3 2×10 @16"o.c. JB210A A E S6 S6 Image: Second s	MFJ1	2x12	@16"o.c.				
MFJ3 2x10 @16"o.c. JB210A	MFB2	4x10 (DROPPED)	N/A			
	MFJ3	2x10	@16"o.c.	JB210A	A E $S6$ $S6$		BATH 1
Image: Sector							
Image: Sector							
Image: Sector							
Image: Market B1 Image: Market B1 <td< td=""><td></td><td></td><td></td><td></td><td></td><td>_</td><td>13'-3"</td></td<>						_	13'-3"
Image: Method set Image: Method set Image: Method set Image: Method set Image: Method set M/A Image: Method set M/A Image: Method set M/A						_	
EMFB1 (E) 4x10 @48"o.c. (DROPPED) N/A N/A						-	
EMFB1 (E) 4x10 @48"o.c. (DROPPED) N/A N/A						_	
EMFB1 (DROPPED) N/A N/A		(E) 4×1	0 @48"o.c.			_	
	EMFB1	(DR)	OPPED)	N/A	N/A	_	
						_	
						_	



Γ		N	ORTH			
-	F	PERM	1IT :	SET		
					02-07-23 PERMIT SET	REV. DATE DESCRIPTION
	ADDITIONS & ALTERATIONS	6020 94th Ave SE Mercer Island, WA 98040		HAUKIAN & SINUHU KNUIZ	6020 3410 AVE SE	Mercer Islana, WA 30040
	CONC.	019810	MIC WASE 52797 RALEN NAL E	REC	COF	
	O.G. ENGINEERING, PLLC Z 3201 1st Ave S Suite 101 Searche WA 98134	(206) 290-4608	owentwogengineer.com SHEET TITLE:	MAIN FLOOR	FRAMING PLAN	
	DOLEDING DEFARIMENT AFFROVAL STAMF: SCALE: BOU BOO	NOTE	SHI	EET. S	^{NO.}	

	P	LAN	LEGEN		_	
		WALL BELOV	V ROOF			
AXA BLW		POST BELOW	$/ \text{ ROOF PER } \underbrace{ \begin{array}{c} E - F \\ S2 \end{array} }$) U.O.N.		
		METAL STRA	P ON OR BELOW	ROOF PER PLAN		
RST1	I	o/ PARALLE FRAMING ME	L BEAM/ JOIST O	F SHEATHING CENTERED R 2x4 FLAT BLKG. ADD S REQ'D TO ALIGN BLW E IN STRAP.		
RST2	2	TOP PLATE.	PLACE OVER WAL	E FACE OF NEW TO (E) DBL LL SHEATHING WHERE OCCUR EQ'D FOR FLUSH UNDERLAY		
LOR1			F LOOKOUT RAFTE	30		
LOR2			LOOKOUT RAFTER		_	· · · · · · · · · · · · · · · · · · ·
EHDF	?	(E) DROPPE	D HEADER OVER \	WALL OPENING BELOW	_	
ELOR BEAM	2	(E) LOOKOU	T RAFTERS			
BEAM HANGER	▲	FRAMING SC	HEDULE FOR HAN (JOIST HANGERS	AM CONNECTION; SEE GERS, U.O.N. ON PLAN NOT SHOWN ON PLAN		
5	Ţ	HEADER (BE WHERE OCC SHOWN FOR	ARING WALL SIM). URS (POST WIDTH	DROPPED BEAM OR POST DOWN TO HEADER TO MATCH BEAM, NOT L FULL-DEPTH BLKG M OVER SUPPORT		LOR1
	FRA	MING	SCHE	DULE		
CALLOUT	JOIST	/BEAM	HANGER (u.o.n. on plan)	REFER TO DETAIL(S	5)	
RR1	2x12		RU212Z (SLOPED) US210 (STRAIGHT			
RB2	BEAM, 1	PSL (RIDGE FOP FLUSH .0. RR1)	N/A	H S6	L	
RH3	3 ¹ / ₂ ×1	I2 GLB D HEADER)	HUCQ412 (TO CORNER POST WHERE OCCURS)	A S2 SIM		
RB4		JSH w/ RR1)	HUS412	N/A		
RB5		1¼ PSL w/ RR1)	HU412 (MANUFACTURED SLOPED)	N/A		
RB6	4x12 (FLU	JSH w/ RR1)	LUS410	N/A		
RH7		(DROPPED ADER)	N/A	A S2		
RH8		DROPPED ADER)	HUC48 (TO CORNER POST WHERE OCCURS)	A S2		K
RT9*	Соммо) ON GABLE S @24"o.c.	N/A	K S6	_	
RT10*	TRUNCA	TED GABLE RUSS	N/A	<u>I K</u> S6 S6	_	
RT11*	STRUCTU	IRAL GABLE TRUSS	N/A	L SPANS OVER WALL S6 OPENINGS BELOW		
ERT1	(E) COM	MON GABLE S @24"o.c.	N/A	N/A		
	- 1					



NOF	RTH
PERMIT	r set
	02-07-23 PERMIT SET DATE DESCRIPTION
	02-07-23 REV. DATE
PROJECT: ADDITIONS & ALTERATIONS 6020 94th Ave SE Mercer Island, WA 98040	CLIENT: HADRIAN & SINDHU KNOTZ 6020 94th Ave SE Mercer Island, WA 98040
ENGINEER	AGAN COLUMN
O.G. ENGINEERING, PLLC B 3201 1st Ave S, Suite 101, Seattle, WA 98134 (206) 290-4608 owen@ogengineer.com	SHEET TITLE: ROOF FRAMING PLAN
SCALE: AS NOTED JOB NO.	sheet. No. S5

